

EQUITATION FACILITY STUDENT BARN AND OUTDOOR ARENA

JONESBORO, ARKANSAS

DR. LESLIE WYATT, PRESIDENT

PROJECT TEAM:

THE CAHOON FIRM, P.A.
ENGINEERING CONSULTANTS, INC.
PETTIT & PETTIT, Consulting Engineers, Inc.

Architecture Structural Engineering

BOARD OF TRUSTEES

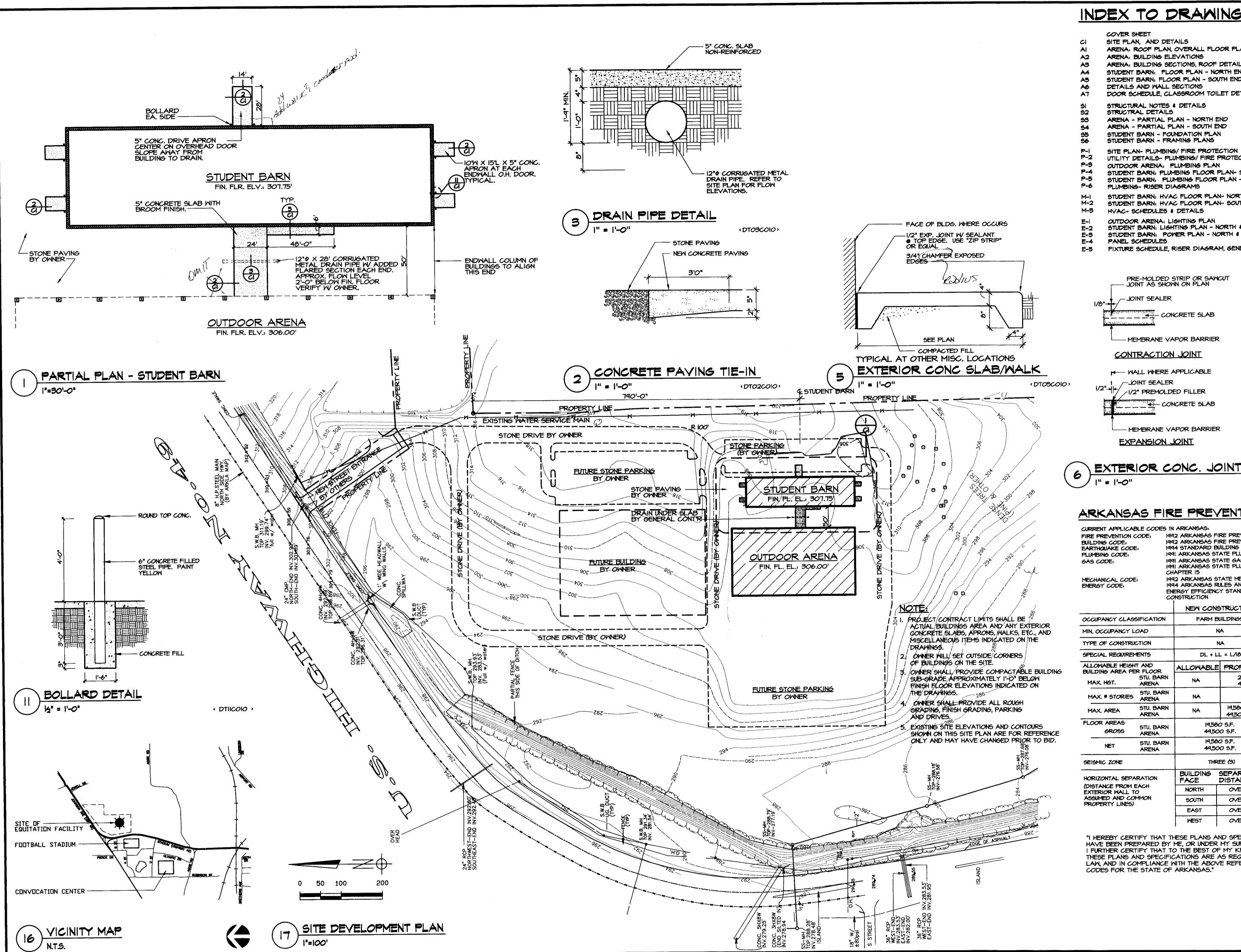
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Chair Vice-Chair Secretary E CAHOON FIRM, P.A.
ARCHITECTURE AND PLANNING

DENT BARN AND ARENA

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Project No.	Date:
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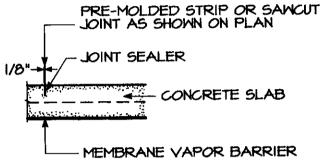
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CONTRACTION JOINT

MALL WHERE APPLICABLE /-JOINT SEALER /1/2" PREMOLDED FILLER ____ CONCRETE SLAB

EXPANSION JOINT

EXTERIOR CONC. JOINT DETAILS

- MEMBRANE VAPOR BARRIER

ARKANSAS FIRE PREVENTION DATA

CURRENT APPLICABLE CODES IN ARKANSAS: FIRE PREVENTION CODE: BUILDING CODE: EARTHQUAKE CODE: PLUMBING CODE:

1992 ARKANSAS FIRE PREVENTION CODE, VOLUME 1 1992 ARKANSAS FIRE PREVENTION CODE, VOLUME II 1994 STANDARD BUILDING CODE 1991 ARKANSAS STATE PLUMBING CODE

MECHANICAL CODE: ENERGY CODE:

1991 ARKANSAS STATE GAS CODE, 1991 ARKANSAS STATE PLUMBING CODE -1992 ARKANSAS STATE MECHANICAL CODE 1994 ARKANSAS RULES AND REGULATIONS FOR ENERGY EFFICIENCY STANDARDS FOR NEW CONSTRUCTION

< DTO6COII

| CODE | REFERENCE NEW CONSTRUCTION FARM BUILDINGS NA 1210.1 DL + LL = L/180 ALLOWABLE PROPOSED

ALLOWABLE HEIGHT AND BUILDING AREA PER FLOOR STU. BARN ARENA TABLE 400 STU. BARN MAX. # STORIES ARENA 14,580 S.F. STU. BARN MAX. AREA ARENA 44,500 S.F. FLOOR AREAS STU. BARN 6R099 49,500 S.F. ARENA STU. BARN 44,500 S.F. ARENA THREE (3) SEISMIC ZONE

BUILDING SEPARATION HORIZONTAL SEPARATION FACE DISTANCE (DISTANCE FROM EACH **OVER 50'** NORTH EXTERIOR WALL TO ASSUMED AND COMMON **OVER 50'** SOUTH PROPERTY LINES) **OVER 50'** EAST

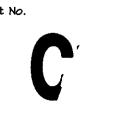
"I HEREBY CERTIFY THAT THESE PLANS AND SPECIFICATIONS HAVE BEEN PREPARED BY ME, OR UNDER MY SUPERVISION. I FURTHER CERTIFY THAT TO THE BEST OF MY KNOWLEDGE THESE PLANS AND SPECIFICATIONS ARE AS REQUIRED BY LAW, AND IN COMPLIANCE WITH THE ABOVE REFERENCED CODES FOR THE STATE OF ARKANSAS."

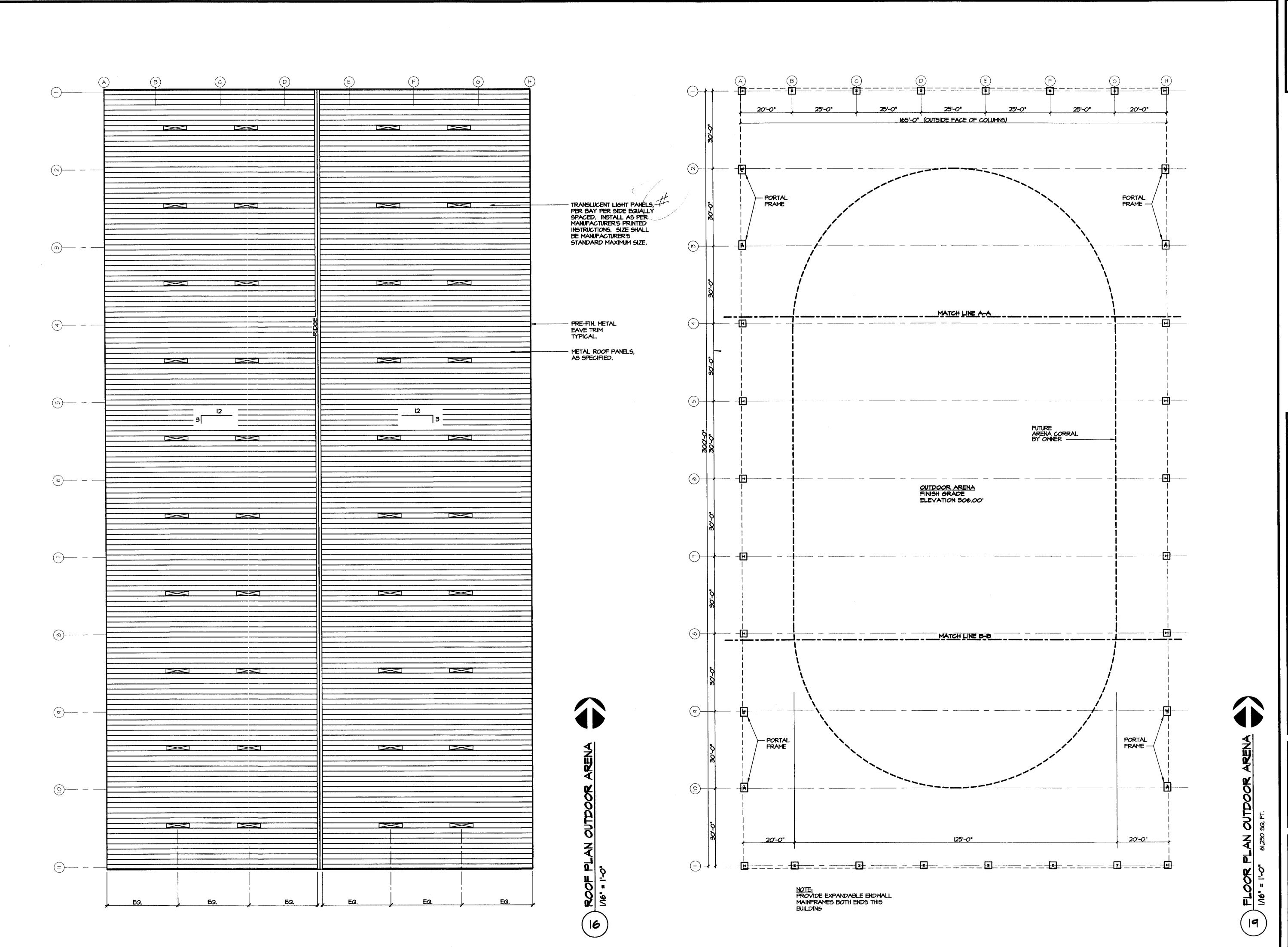
OVER 50'

TABLE 600

Sheet Name SITE PLAN Sheet No.

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ENT BARN AND ARENA

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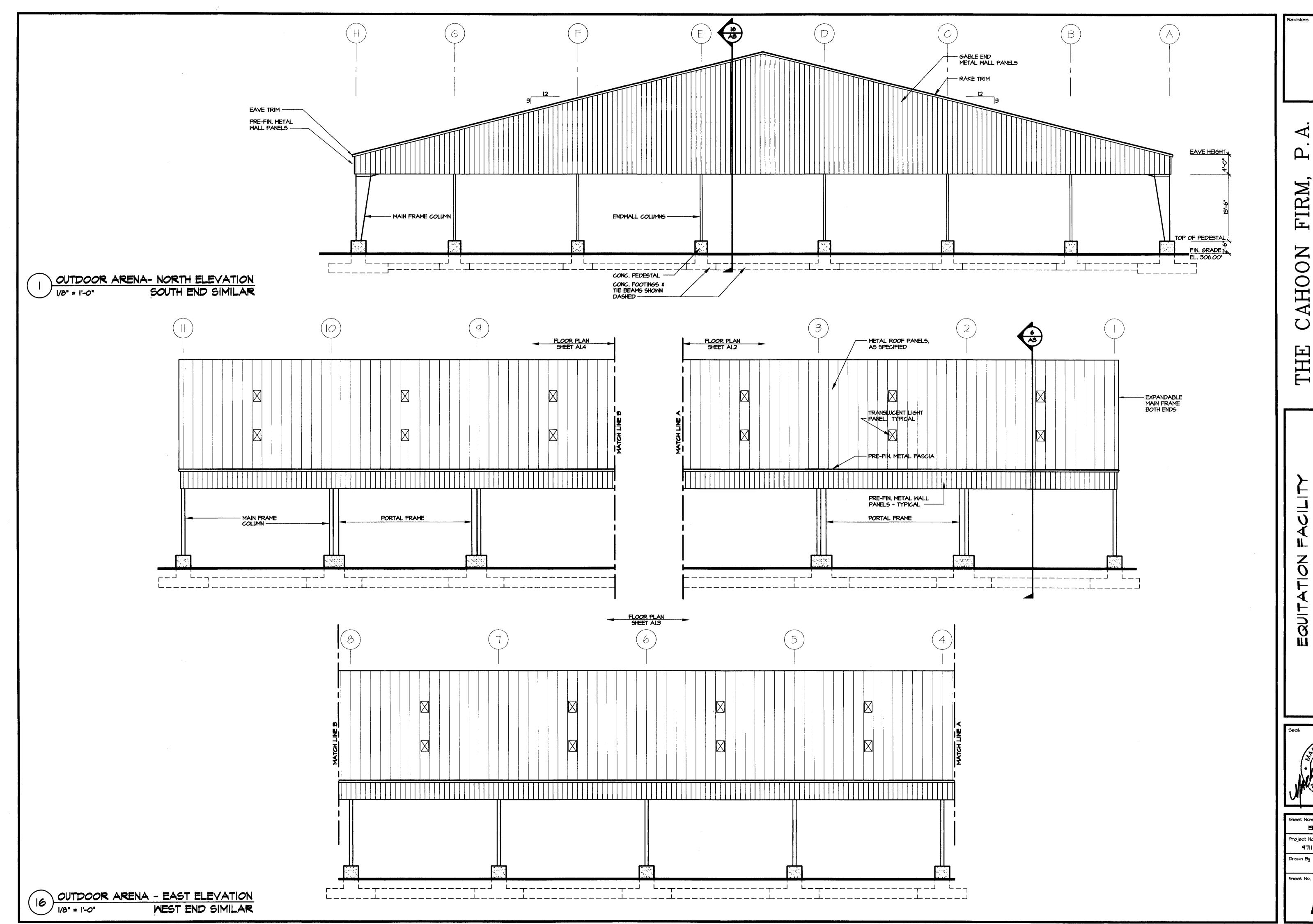
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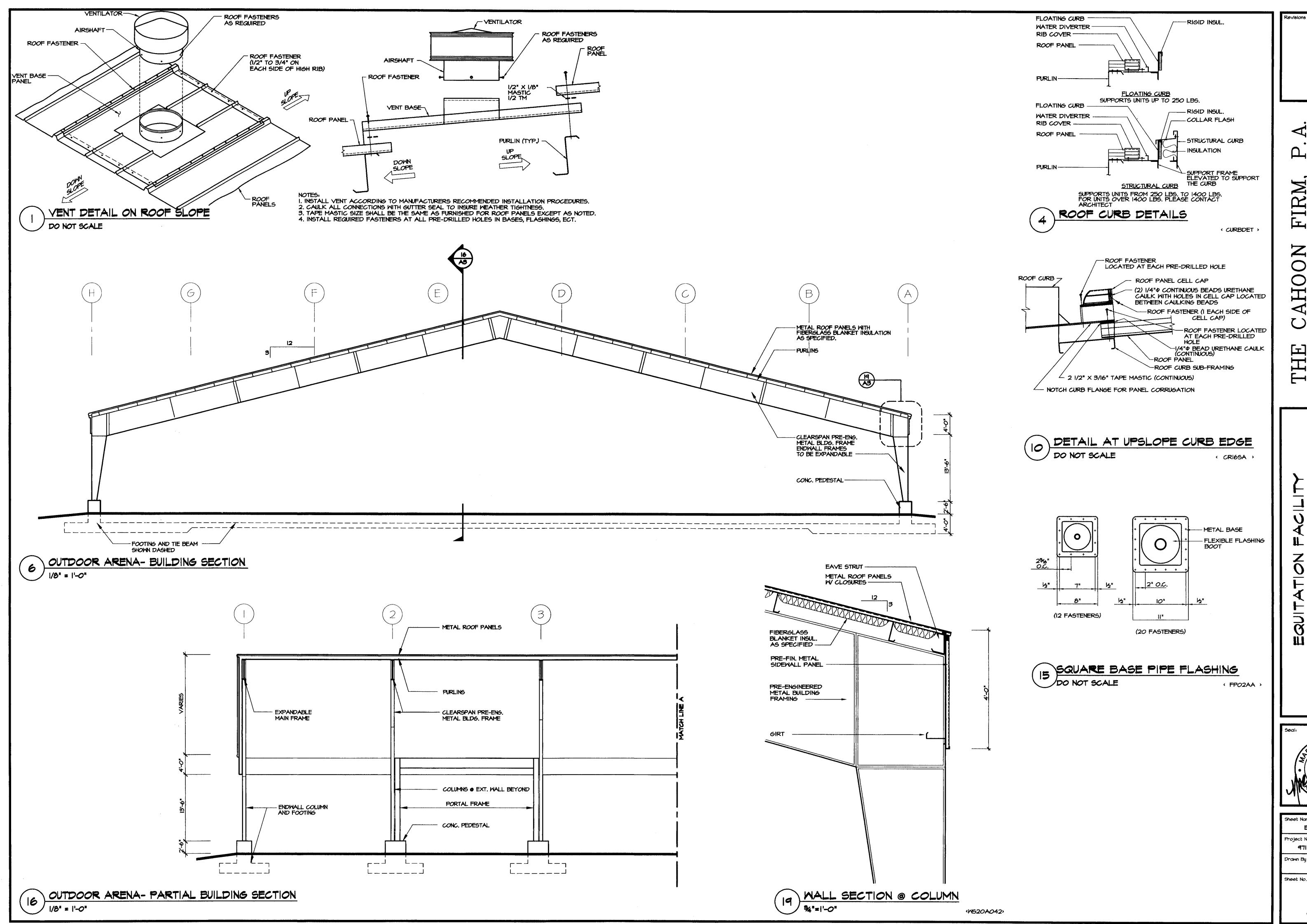
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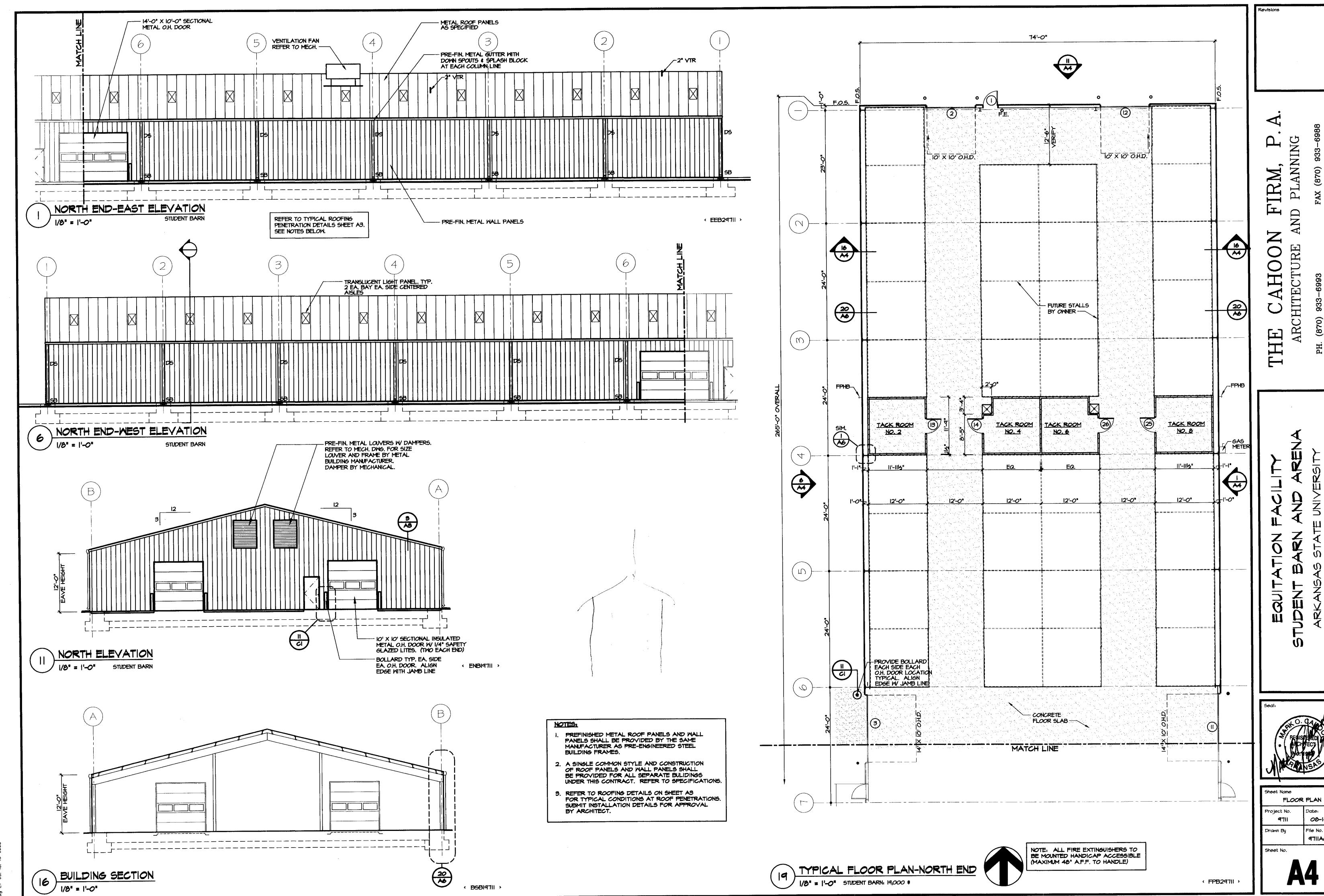


ELEVATIONS Project No.

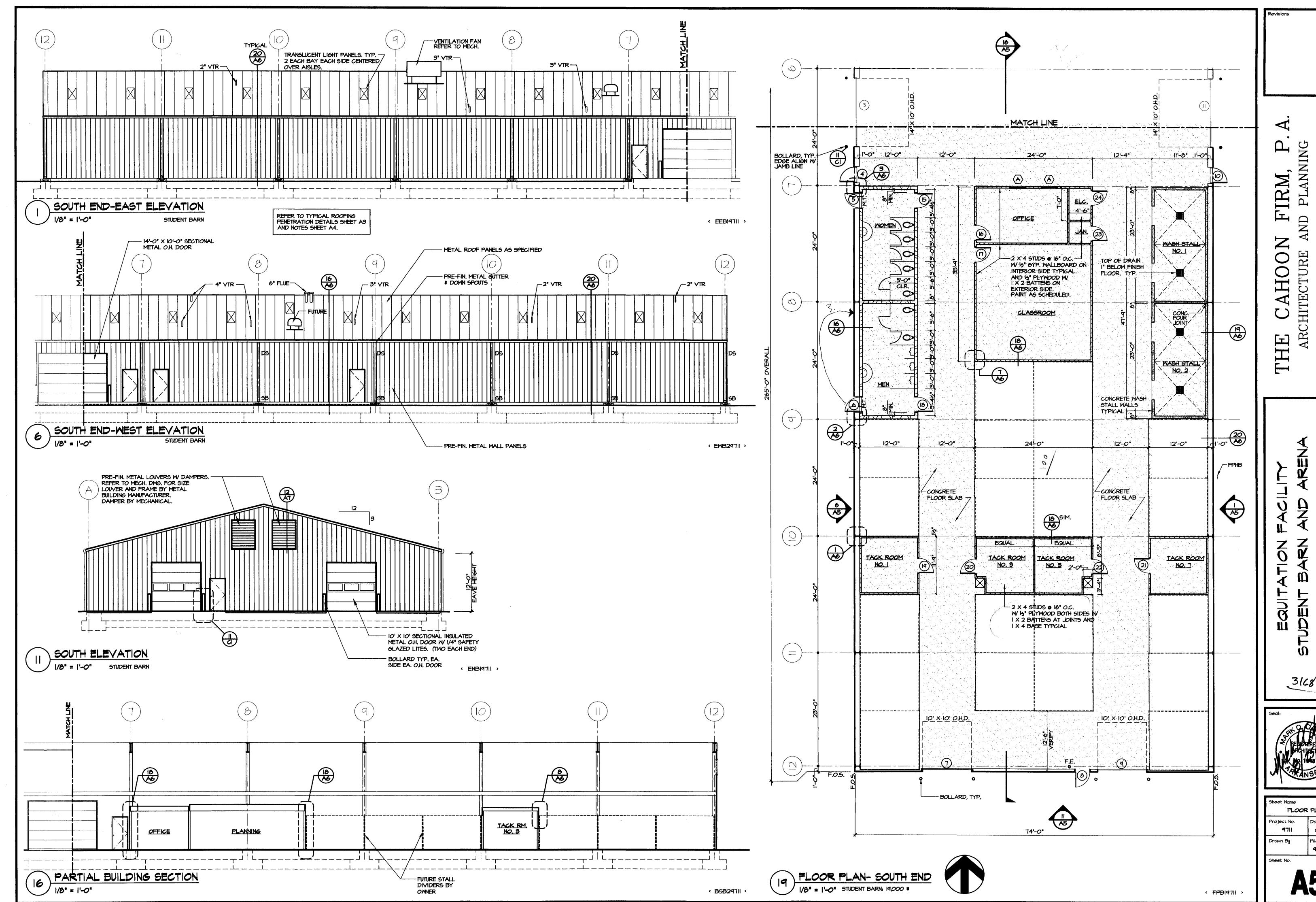


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ELEVATIONS 08-14-98 Drawn By 9711A030

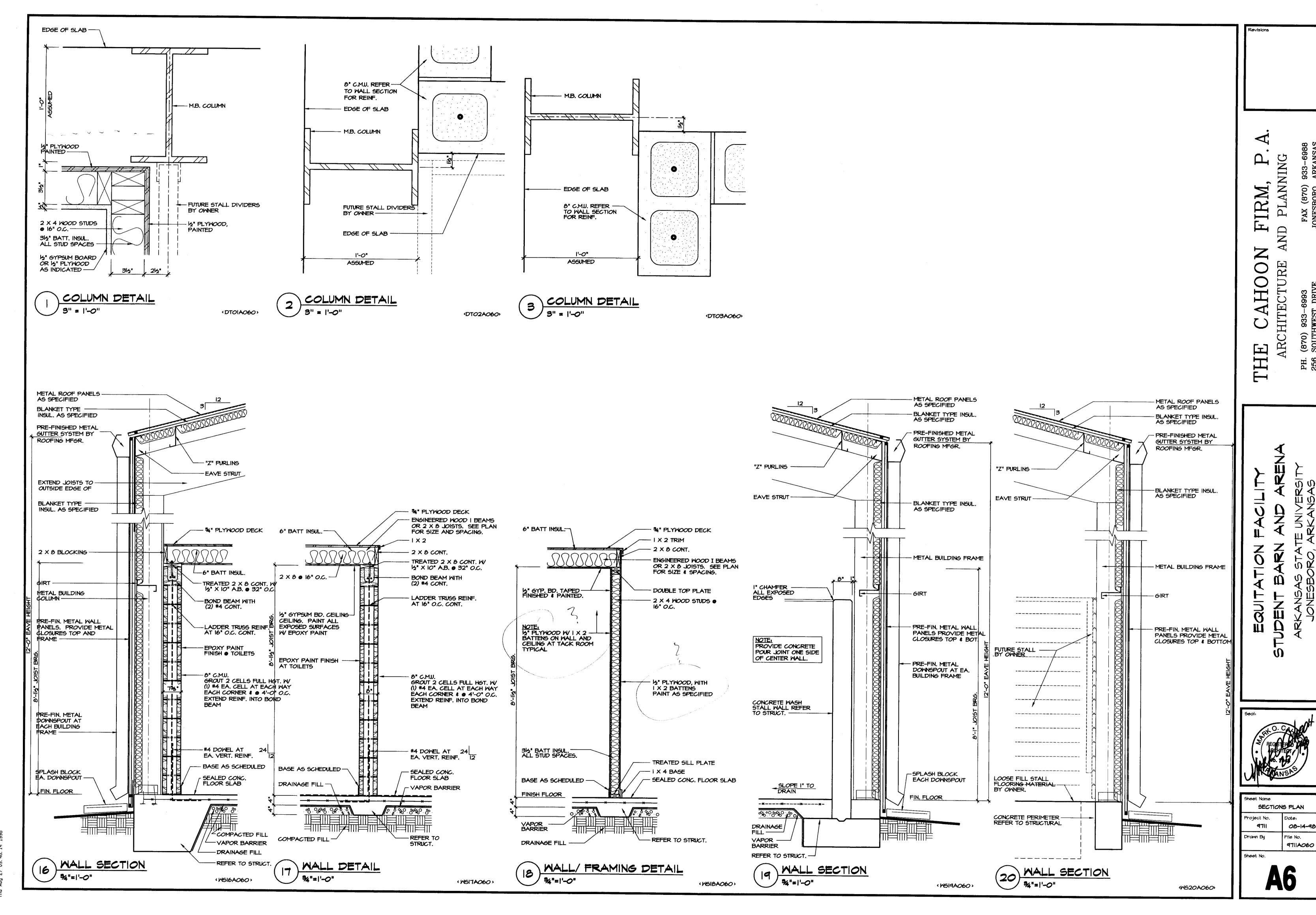


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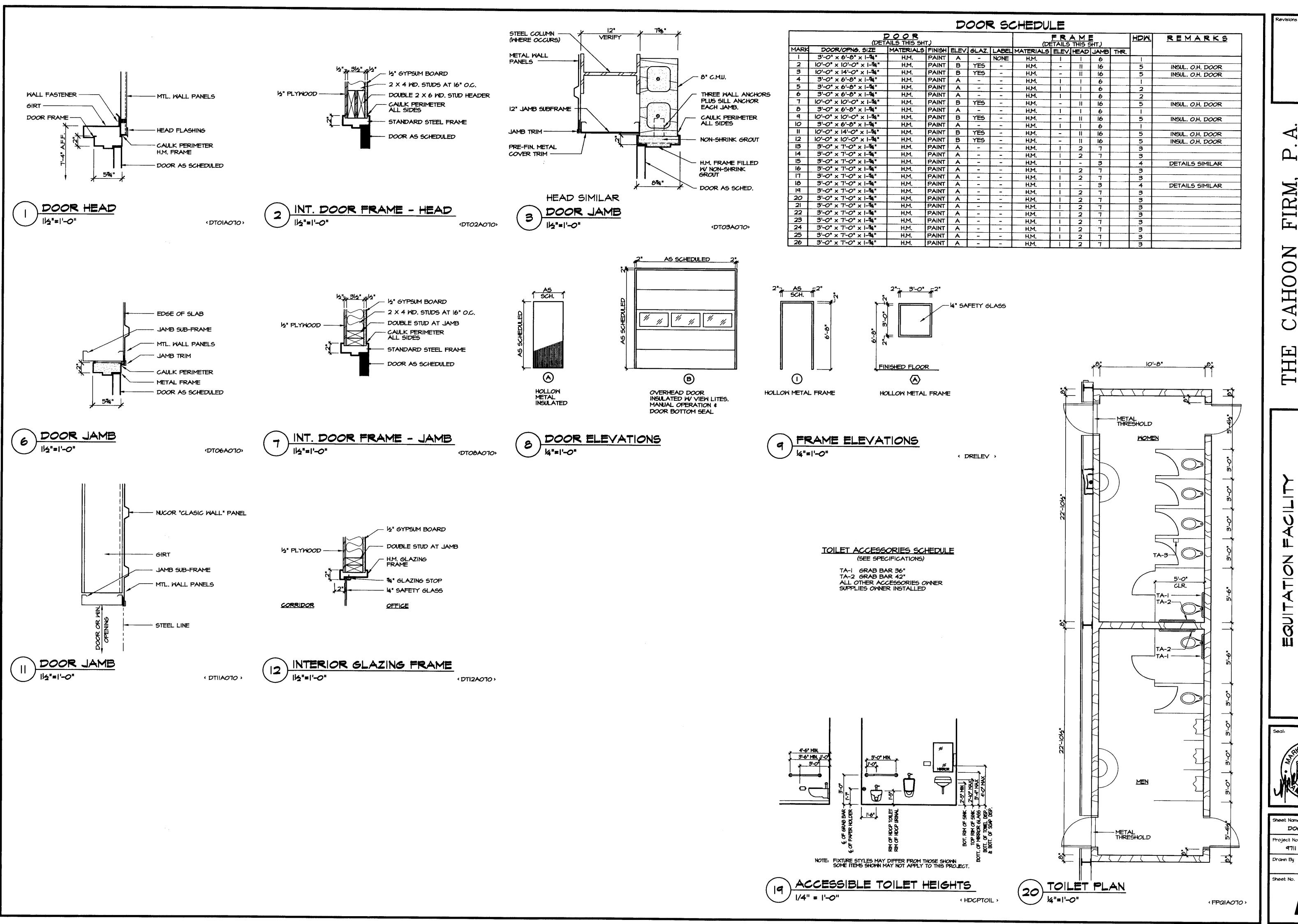


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E CAHOON FIRM, P. A ARCHITECTURE AND PLANNING

PH. (870) 933-6993 256 SOUTHWEST DRIVE

STUDENT BARN AND ARENA
ARKANSAS STATE UNIVERSITY
JONESBORO, ARKANSAS

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DOOR DETAILS

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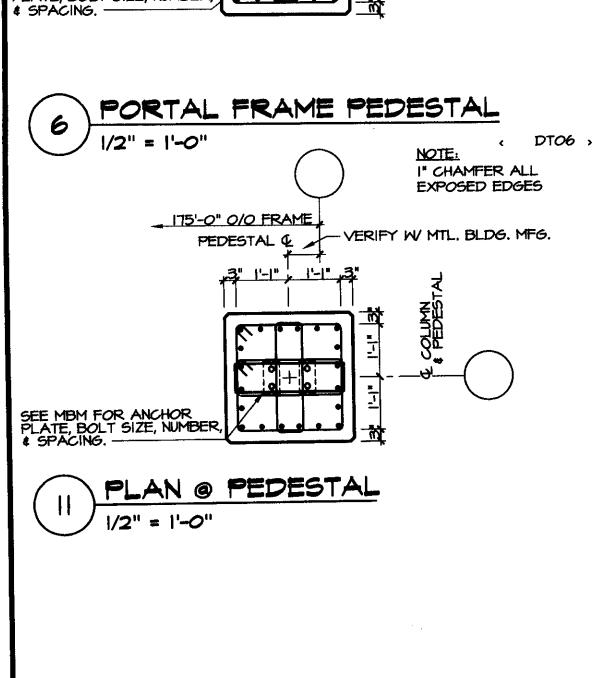
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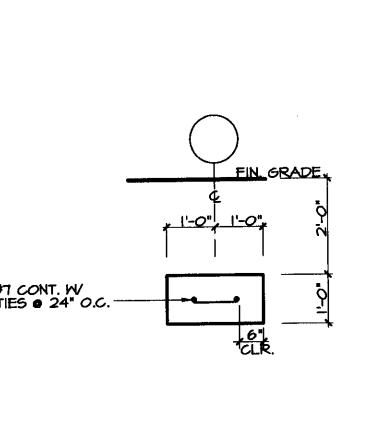
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GENERAL NOTES In case of conflict between the General Notes below and the specifications the more rigid requirements shall govern unless amended in writing by the Engineer. Site selection, evaluation, and preparation is not the responsibility of the Engineer. ACI specifications shall govern all phases of fabrication and construction. DESIGN DATA Design Codes - (All latest editions unless noted) -American Concrete Institute (ACI) -American Institute of Steel Construction (AISC) -American Welding Society (AMS) -American Iron and Steel Institute Specifications for Design of Cold Formed Steel Structural Members -Southern Standard Building Code (SSBC) - 1994 Edition -American Society of Civil Engineers (ASCE 7-88 formérly ANSI A58.1-1982) Minimum Design Loads for Buildings and Other Structures . Material Specifications and Design Stresses -Anchor Bolts ASTM A36 -Embedded Steel -Cast-in-place Concrete: all concrete shall have a minimum compressive strength of at least f'c = 3,000 psi at 28 days. All concrete exposed to weather shall have 51/3% ± 11/3% air entrainment. Total entrapped air shall not exceed 3% for interior slabs with steel trowel finish. -Reinforcing Steel ASTM A615, Grade 60 -Welded Plain Wire Fabric 6x6/W2.9xW2.9 SHEET MESH ONLY I" CHAMFER ALL EXPOSED EDGES 175'-0" 0/0 FRAME VERIFY W MTL. BLDG. MFG. PEDESTAL & SEE MBM FOR ANCHOR PLATE, BOLT SIZE, NUMBER & SPACING.





2- #7 CONT. W #3 TIES @ 24" O.C. 1/2" = 1'-0" GENERAL NOTES (CONTINUED)

3. Design Soil Bearing Pressures -Footings on natural soils are designed for an assumed total contact bearing pressure of 2,000 psf at bottom of footings. -Footings on compacted engineered fill are designed for a maximum soil bearing pressure of 2,000 psf. (FIELD VERIFY) -if the soil at the footing bearing elevations shown is of questionable bearing value, the Engineer or Architect shall be notified immediately. -After Footing excavations are completed and before placing concrete, the excavated areas shall be inspected and approved by the Owner selected independent testing laboratory as specified.

4. Design Loads -Column reactions as provided by pre-engineered building mfg. Nucor Building systems, 305 industrial Parknay, Waterloo, In. -Seismic Zone 3 Z = 0.75, K = 1.33, C5 = 0.14 max. 1 = 1.00, 5 = 1.50, C = 0.12 max.

5. Seismic Foundation Design The foundations have been designed to resist seismic forces per the 1994 Standard Building Code in accordance with the requirements of Act 1100 of the 1991 Arkansas State Legislature. The required seismic design data is as follows:

SEISMIC ZONE PER ACT 1100 OF 19991 FOR THE STRUCTURE: PEAK VELOCITY RELATED ACCELERATION AV (1607.15):. ..0.25 PEAK ACCELERATION Aa (1607.1.5): SEISMIC HAZARD EXPOSURE GROUP (1607.1.6): SEISMIC PERFORMANCE CATEGORY (1607.1.8): SOIL PROFILE TYPE (1607.3.1):. BASIC STRUCTURAL SYSTEM (1607.3.3): SEISMIC RESISTING SYSTEM (1607.3.3)......ORDINARY FRAMES OF STEEL RESPONSE MODIFICATION FACTOR R (1607.3.3)... DEFLECTION AMPLIFICATION FACTOR, Cd (1607.3.3): ...1607.4 ANALYSIS PROCEDURE UTILIZED (1607.4 OR 1607.5):.

Seismic structural design for metal building frames and components shall be performed by others.

GENERAL INFORMATION

SEE MBM FOR ANCHOR PLATE, BOLT SIZE, NUMBER & SPACING.

1. In cases of discrepancies between dimensions and elevations between structural and architectural drawings, contractor shall coordinate with architect prior to fabrication and construction.

2. All columns shall be centered on arid lines unless noted otherwise. 3. All column footings shall be centered on columns unless noted

4. All wall footings shall be centered on walls unless noted otherwise. 5. Unless otherwise noted or detailed, concrete pads for mechanical equipment shall be 4" thick (minimum) and reinforced with

#3 at 12" o.c. each way centered. 6. Substitution of expansion anchors for embedded anchors shall not be

7. Back fill both sides of all foundation and retaining walls equally until low side is up to the finish grade. Do not back fill any walls until concrete has reached its specified 28-day compressive strength. 8. A 6-mil polyethylene film vapor barrier shall be placed below all

Interior slabs-on-grade. 9. Provide a 4" clean medium to coarse sand or gravel compacted drainage fill below all interior slabs-on-grade unless noted or detailed otherwise.

10. Contractor shall provide temporary guys and bracing as required during construction. Structure is not stable until all structural members connections, and decking are in place. See metal building manufacturer for special erection requirements.

I" CHAMFER ALL

EXPOSED EDGES

- $ar{ullet}$ VERIFY W MTL. BLDG. MFG.

O/S FRAME

11. Damaged areas of hotdip galvanizing on anchor bolts, plates, etc. shall be repaired with two (2) coats of cold spray galvanizing.

FOUNDATION NOTES

1. Spread footings shall bear at least 3'-4" below lowest adjacent finished grade on stiff undisturbed natural soil or properly compacted

select fill with an allowable net bearing pressure of at least 2000 PSF. 2. Prior to placing fill, completely remove all organic containing soils, highly plastic soils, miscellaneous debris, roots, old foundations, slabs, walls, etc. and all buried tanks, vaults, manholes, etc. not specifically identified to remain from within ten feet of the building perimeter.

3. After stripping, the area within ten feet of the building perimeter shall be proof rolled and all soft zones shall be reworked or undercut and replaced with properly compacted low plasticity select fill.

4. All undercutting, site preparation, fill selection, back filling and compaction

shall be performed in strict accordance with the soils engineer's recommendations. Refer to soils report prepared by Mid-Continent Laboratories, Inc. dated Nov. 19, 1997, included in contract specifications. 5. All spread footing excavations shall be inspected by the soils engineer

or his representative to verify that the design net bearing capacity is attained. 6. All fill up to the top of the footing elevation within five feet of the building perimeter shall be compacted to at least 95% of modified

proctor maximum dry density (ASTM DI557). 7. If old basements or other deep existing structures are discovered within ten feet of the building perimeter, notify architect immediately.

CAST-IN-PLACE CONCRETE

1. Arrangement and bending of reinforcing steel shall be in accordance with ACI detailing manual, latest edition. 2. Reinforcing steel shall be new and all bars shall be deformed.

3. Where reinforcing bars are shown continuous, lap bars 36-bar diameters or 24-bar diameters at tension or compression splices respectively (12" minimum). Splice tension ties with 125% tension capacity mechanical couplers.

Do Not loo splice tension ties. 4. Provide suitable wire spacers, chairs, ties, etc., for supporting reinforcing steel in the proper position while placing concrete. Brick chairs are not acceptable

5. Concrete protective covering for reinforcement at surfaces not exposed directly to the ground shall be 3/4" for slabs, joists, and walls and 1-1/2" for beam stirrups and column ties or spirals. 6. Concrete protective covering for reinforcement at surfaces which will

be exposed to the meather or be in contact with the ground shall be 2" for bars larger than #5 and 1-1/2" for #5 bars and smaller. Provide 3" cover below and at ends of footing bars. 7. Location and sizes of openings, sleeves, etc., required for other trades

must be verified by these trades before placing concrete.

PRE-ENGINEERED METAL BUILDING

The building manufacturer must be a current AISC member. 2. The building shall be a manufacturer's standard prefabricated metal structure of the approximate inside area shown, except as noted. Rigid frames shall be spaced as shown on the plans, but overall dimensions and construction details may vary to suit manufacturer's standard design. Minimum web thickness of rigid frames shall be

3. The building shall be designed and fabricated according to AISC, MBMA, and AISI latest specifications. The dimensional tolerances outlined in the Ah15 code under workmanship and the tolerances applicable to roll form steel under the AISC "STANDARD MILL PRACTICE" section shall be required in the fabrication of the steel building frames.

4. The building frame shall be designed to limit the lateral deflection in inches to 0.0042 times the lower eave helpht for the governing basic wind speed of 70 mph.

5. A complete design analysis showing all calculations for the rigid frames, girts, purilins, and x-bracing for wind and zone 3 seismic loads and layout of anchor bolts and other embedded Items shall be submitted for the approval with the shop drawings. Shop drawings shall include details of all main members, typical connections (showing bolt holes and welds), and erection drawings.

6. The building shall be designed to support all mechanical equipment including heaters, sprinklers, exhaust systems, service equipment, and all other such devices. Additional girts or purins shall be placed in convenient locations for attachment of all mechanical equipment.

7. Metal building vertical bracing shall consist of portal frames at locations shown on plans. Metal building manufacturer shall coordinate location of all braces to minimize interference with architectural features. Rod or cable braces may not be substituted where portal frames are shown.

8. Combination design loads conditions shall comply with MBMA specifications.

9. Maximum purlin live load deflection shall not exceed SPAN/240.

10. Frame live load deflection shall not exceed SPAN/360.

11. Maximum girt lateral deflection from wind or selemic loads shall not exceed SPAN/240 for girts providing lateral support for

12. Maximum building side sway (drift) from wind or seismic loads shall not exceed wall HEIGHT/240.

13. Metal building framing layout and members shown are suggested only. Manufacturer is responsible for coordinating requirements with owner and providing complete structural framing system designed by the manufacturer. Metal building manufacturer shall coordinate all dimensions, elevations, bracing and sizes and shapes of members with owner prior to fabrication and construction. All member connections and decking not specifically sized on drawings shall be designed and supplied by the metal building manufacturer.

14. Metal building manufacturer shall provide shop drawings & calculations, stamped by a Professional Structural Engineer registered in the state of Arkansas, for review prior to fabrication.

EXISTING CONSTRUCTION

1. Before fabrication and erection of any materials, field verify all existing elevations, dimensions, and other conditions as shown on the drawings and report any discrepancies to the Engineer or Architect

ROOF FRAMING

1. Roof framing structure including main frames, end wall framing and roof purlins shall meet the following requirements.

A) All purlin bracing required for non diaphragm 5.5. roof sheets to be designed & furnished by building supplier.

B) DESIGN LOADS:

ROOF DEAD LOAD: Actual weight of roof plus hanging equipment, lights, etc. (4 psf min. collateral dead load. Total dead load used for design shall be at least 7 psf)

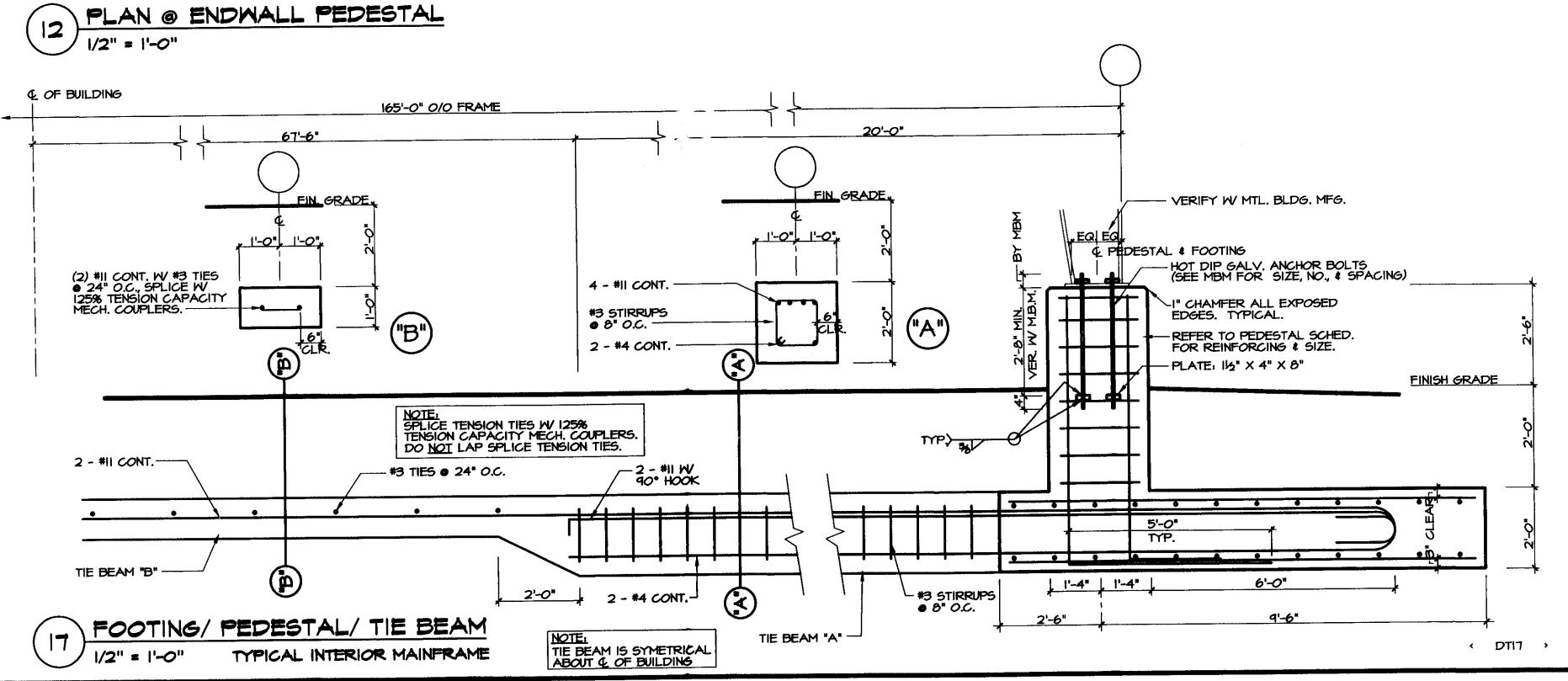
ROOF LIVE LOAD: 20 PSF (purlins and frames). Live load reductions for frames will not be allowed.

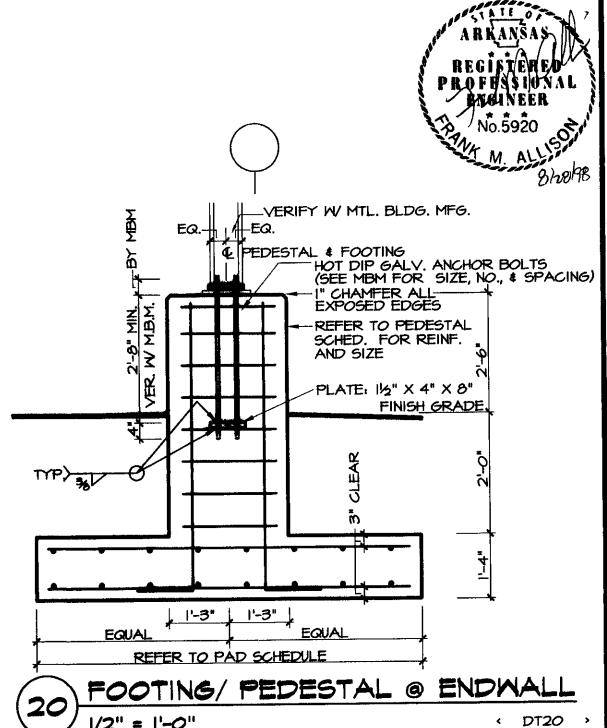
WIND: 70 mph SEISMIC: ZONE 3

CODES: 1994 Standard Bullding Code MBMA Metal Building Systems Manual (latest edition)

PAD FOOTING SCHEDULE												
MARK	PAD SIZE	THICKNESS	REINFORCING	REMARKS								
FI	12'-0" X 10'-0"	2'-0"	#8 ● 12" O.C.	EACH WAY TOP & BOTTOM								
F2	12'-0" X 12'-0"	2'-0"	#8 • 12" O.C.	EACH WAY TOP & BOTTOM								
F3	8'-0" × 6'-0"	1'-4"	#7 • 12" O.C.	EACH WAY TOP & BOTTOM								
F4	6'-0" × 6'-0"	1'-4"	#6 • 12" O.C.	EACH WAY TOP & BOTTOM								
F5	5'-0" × 5'-0"	l' -4 "	#6 e 2" O.C.	EACH WAY TOP & BOTTOM								

PEDESTAL SCHEDULE										
MARK	SIZE	VERT. REINFORCING	TIES							
PI	2'-8" × 2'-8"	20- #9	#4 TIES @ 8" O.C.							
P2	2'-8" × 3'-4"	22- #9	#4 TIES @ 8" O.C.							
P3	2'-8" X 2'-8"] 12- #9	#4 TIES ● 8" O.C.							
P4	-6" × -6" [8- #6	#4 TIES • 8" O.C.							

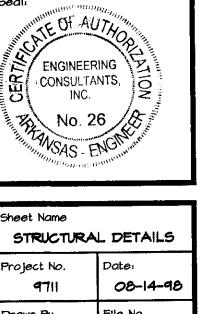




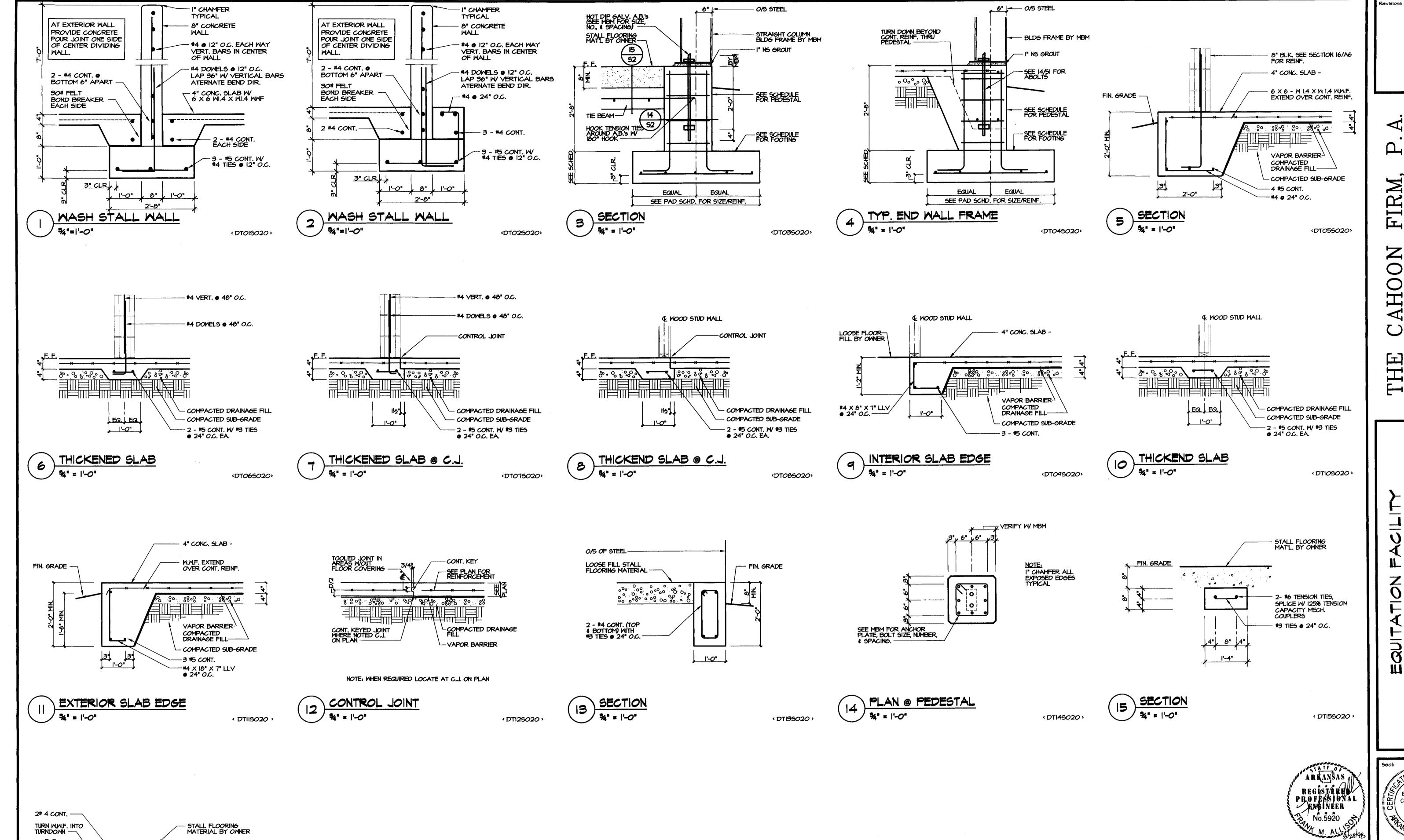


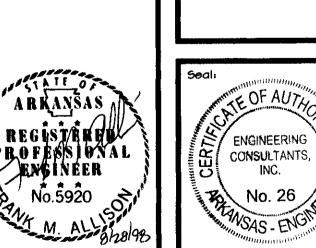
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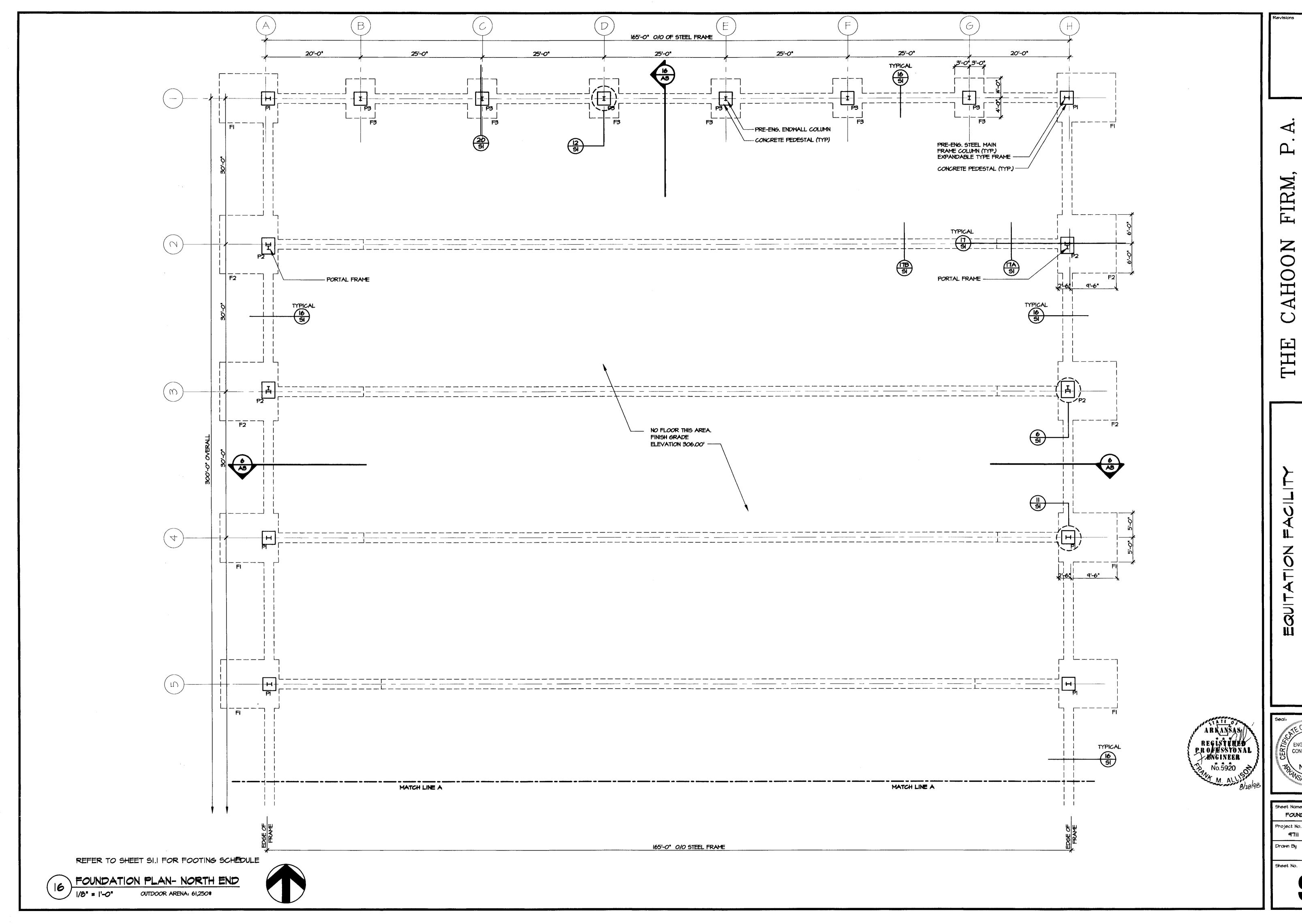
SECTION

|'-4"

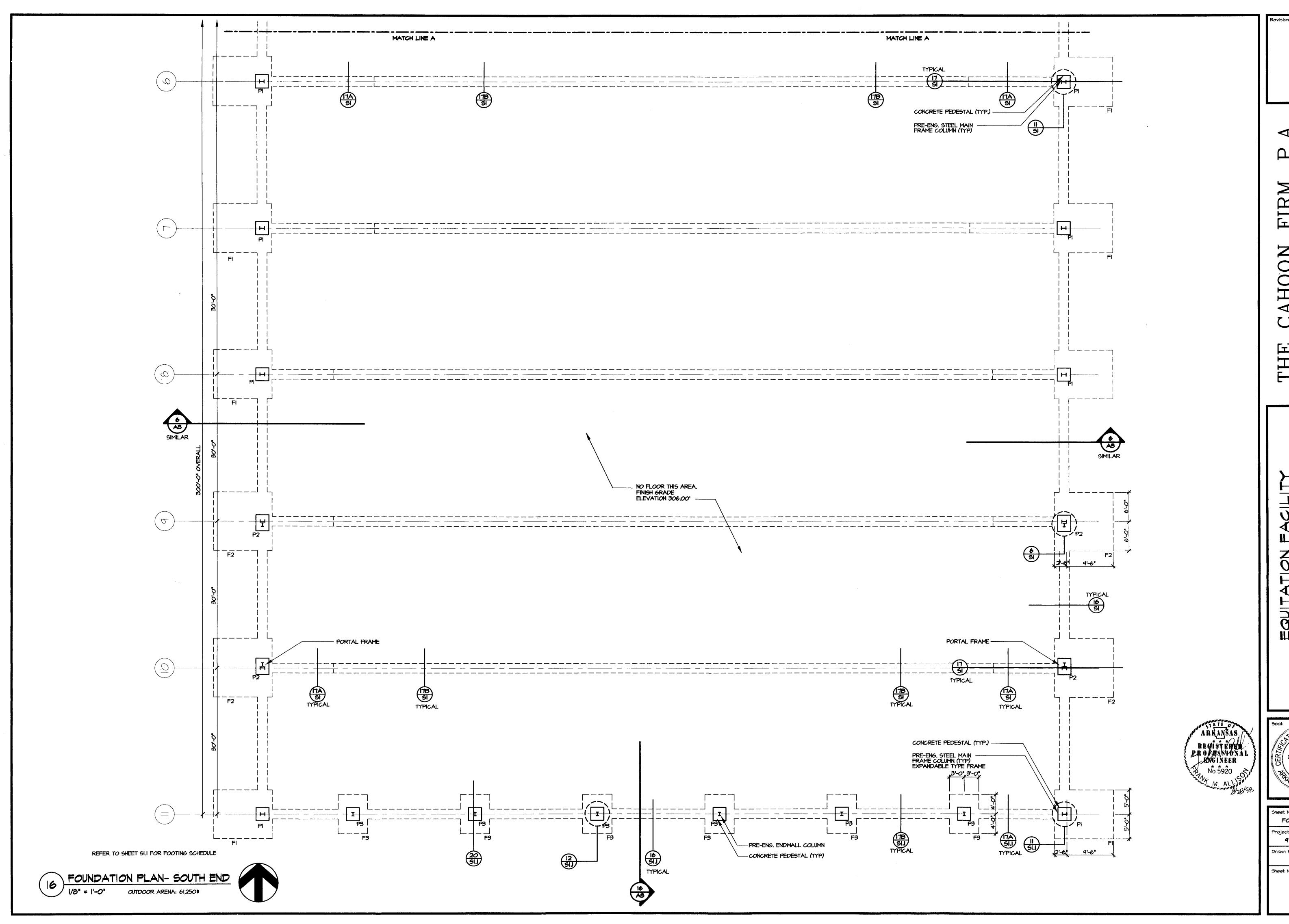
-2 - #6 TENSION TIES, SPLICE W 125% TENSION CAPACITY MECH. COUPLES

<DTI65020>

- #3 TIES ● 24" O.C.



FOUNDATION PLAN



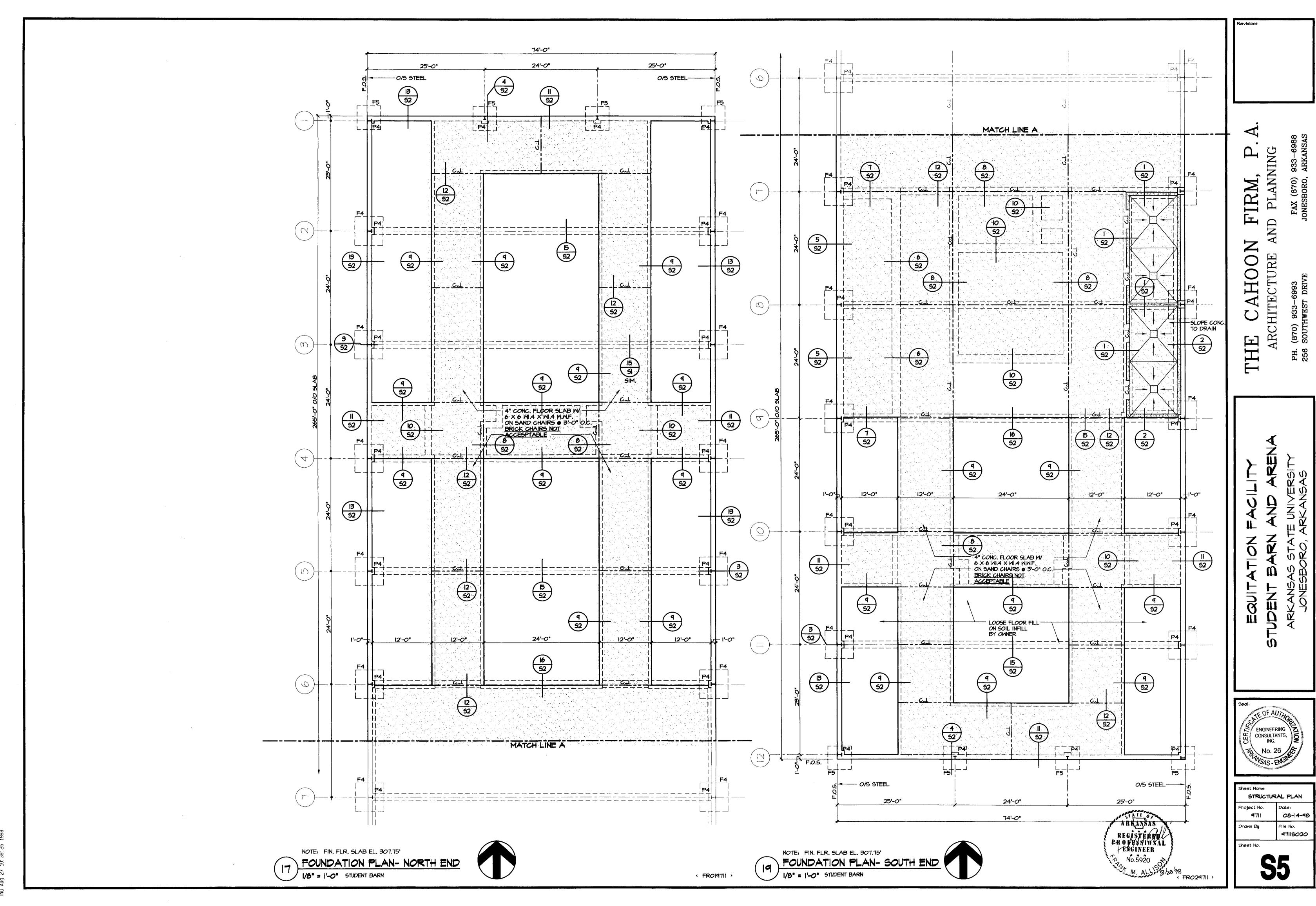
THE CAHOON FIRM, P.A. ARCHITECTURE AND PLANNING

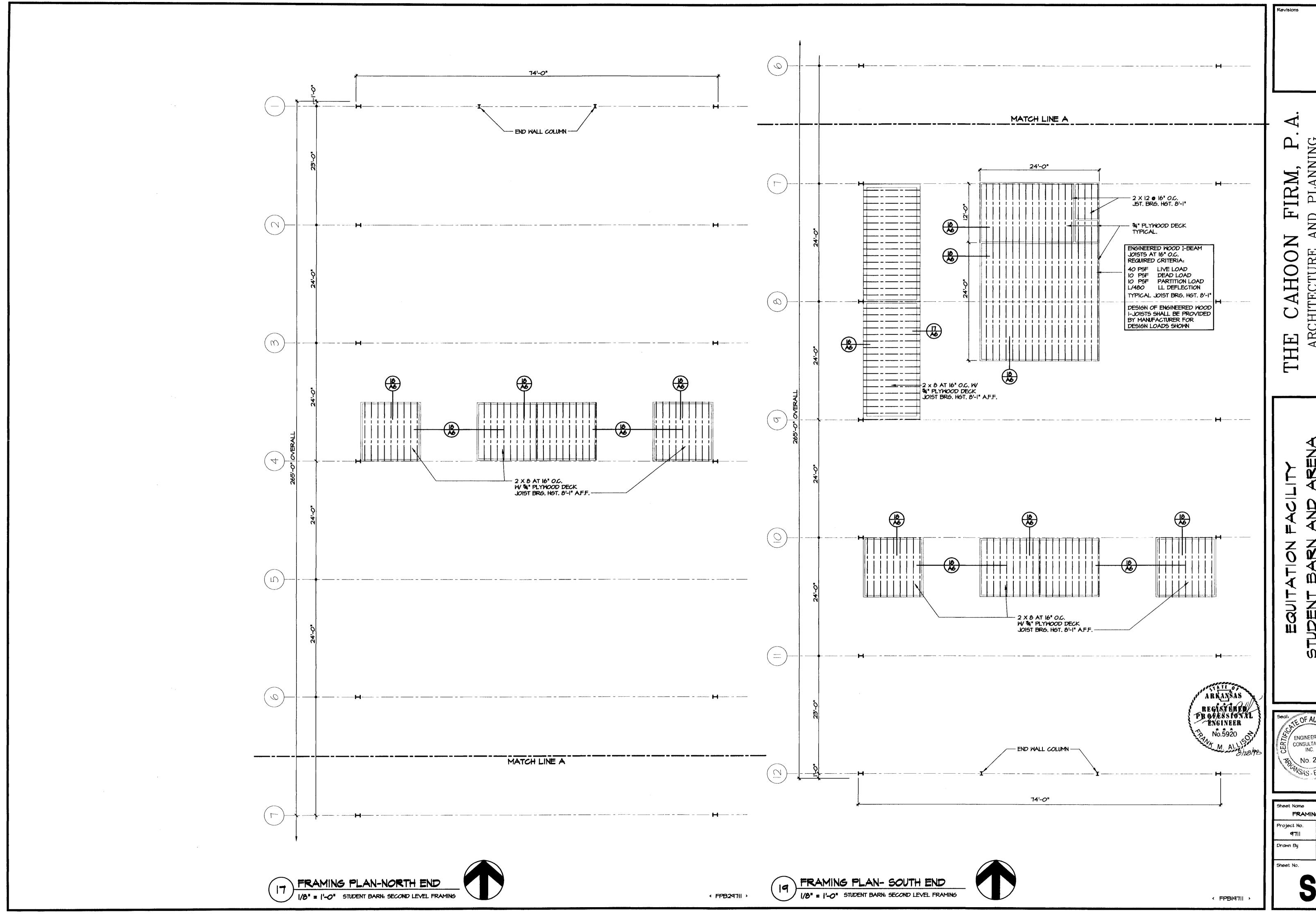
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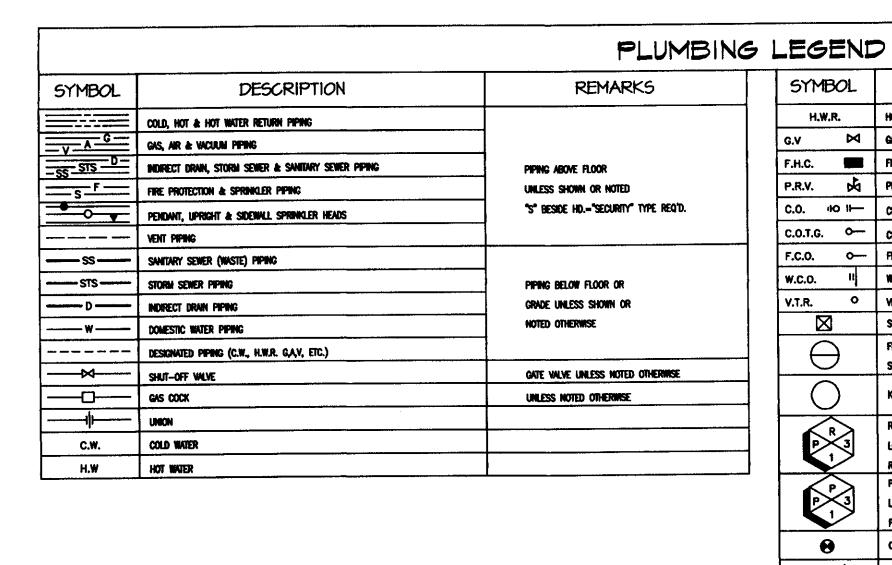
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IKIMI, F. PLANNING ARCHITECTURE

FRAMING PLAN 08-14-98 97115060



SYMBOL	DESCRIPTION	REMARKS
H.W.R.	HOT WATER RETURN	
G.V ⋈	GATE VALVE	
F.H.C.	FIRE HOSE CABINET	
P.R.V.	PRESSURE REDUCING VALVE	
C.O. 40 1⊢	CLEANOUT EXOPSED	SIDE OF PIPE RISE OR END OF PIPE RU
C.O.T.G. 0-	CLEANOUT TO GRADE	SEE DETAIL
F.C.O. O-	FLOOR CLEANOUT	
W.C.O. 1	WALL CLEANOUT	
v.t.r. o	VENT THRU ROOF	see general notes
\boxtimes	SHJT-OFF VALVE IN BOX	
	FIRE PROTECTION SYSTEM	
	STANDPIPE RISER NUMBER	
	KITCHEN EQUIPMENT REFERENCE NUMBER	SEE KIT. EQUIP. DRAWINGS
R	RISER DIAGRAM	
P 3	LOCATION SHEET NUMBER	
	RISER DIAGRAM MUMBER	
	PARTIAL PLAN LOCATION SHEET NUMBER	
	PARTIAL PLAN NUMBER	
8	CONNECT TO EXISTING	
L #	THRUST BLOCK	SEE DETAIL

KEYED NOTES

REFERENCE GENERAL NOTES & LEGEND ON THIS SHEET.

PROVIDE WATER METER, SETTING,
VALVING, BY-PASS, ETC. IN
STRICT ACCORDANCE WITH CITY
WATER & LIGHT CO. ALL COST
BY PLUMBING CONTRACTOR.

3 ESTIMATED WATER DEMAND= 90 GPM

4 ESTIMATED WATER DEMAND= 128 GPM

D

Revisions

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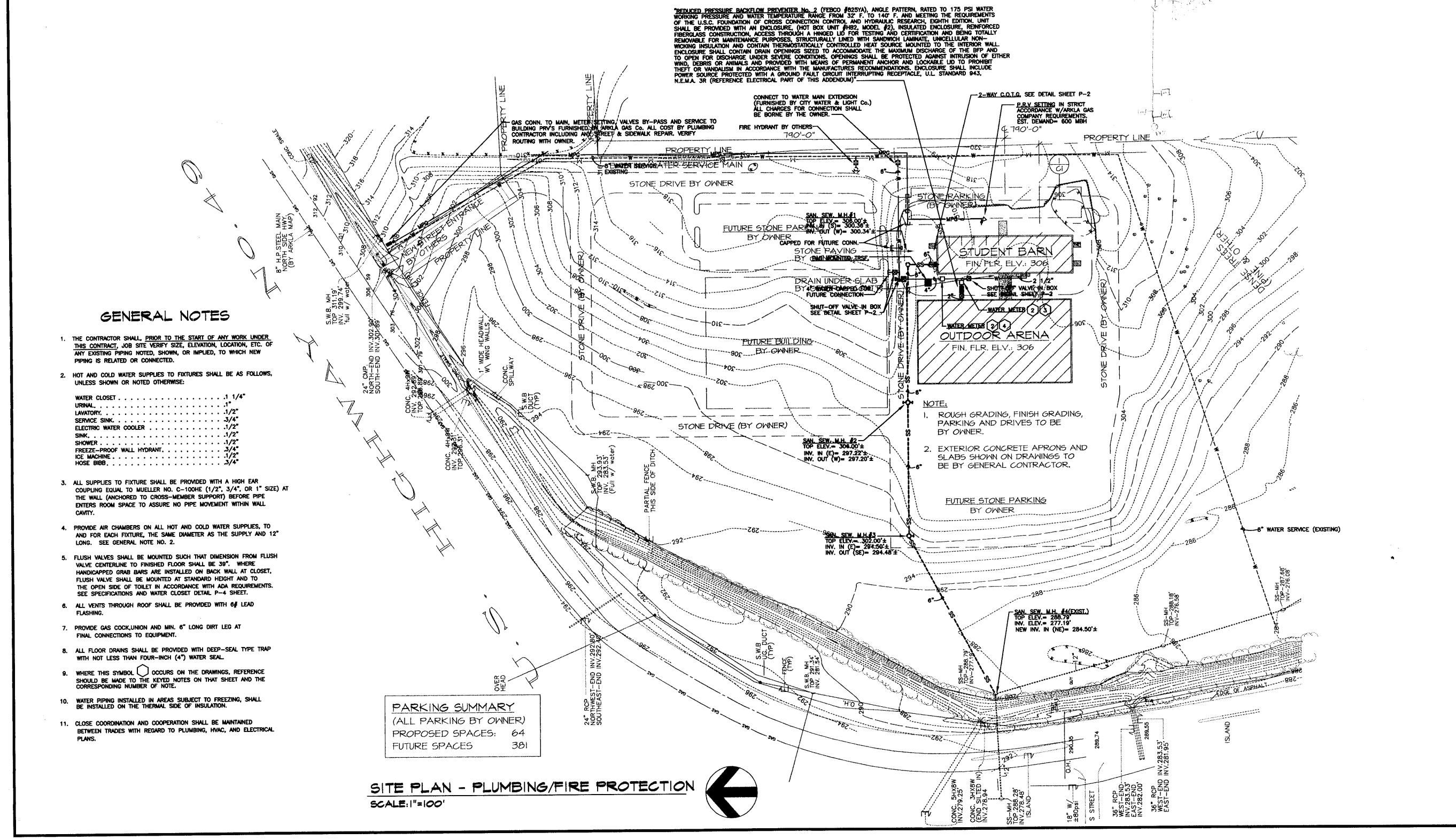
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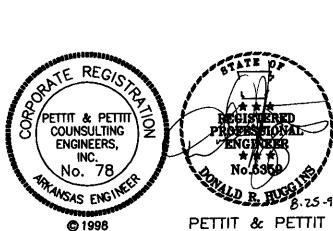
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FIRE PROTECTION

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8-14-48

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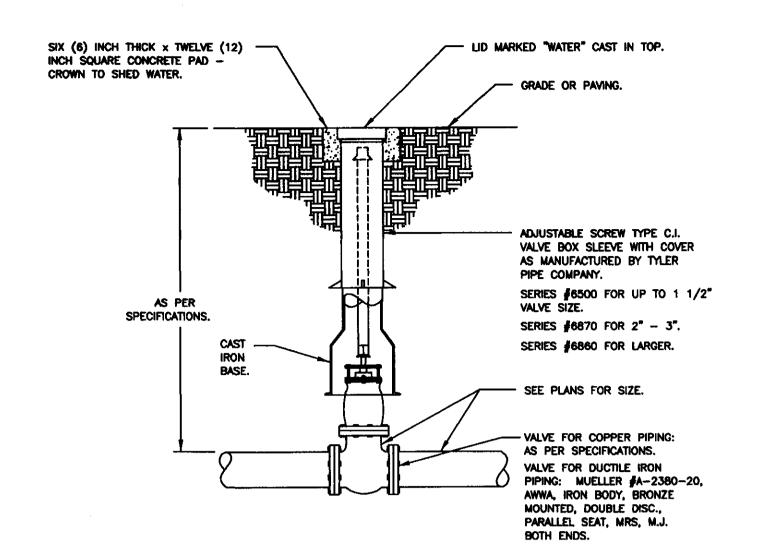


CONSULTING ENGINEERS, INC.

LITTLE ROCK, ARKANSAS

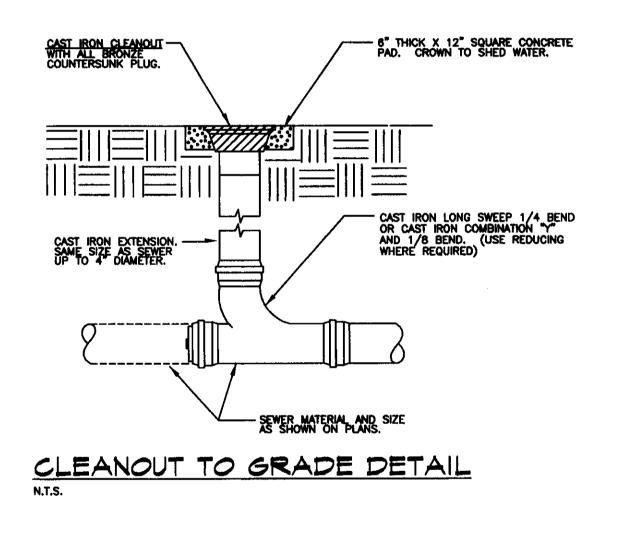
PETTIT & PETTIT CONSULTING ENGNEERS, INC.

REFERENCE GENERAL NOTES & LEGEND ON SHEET P-1.



VALVE BOX DETAIL
NOT TO SCALE

NOTE: CONTRACTOR SHALL PROVIDE TO THE OWNER UPON COMPLETION OF THIS WORK, A VALVE OPERATING WRENCH WITH SOCKET SIZE TO FIT VALVE NUT AND SUFFICIENT LENGTH TO PROVIDE "T" CENTERLINE AT FORTY—TWO (42) INCHES ABOVE FINISH GRADE. WRENCH SHALL BE EQUAL TO #A-24610 AS MANUFACTURED BY MUELLER COMPANY.



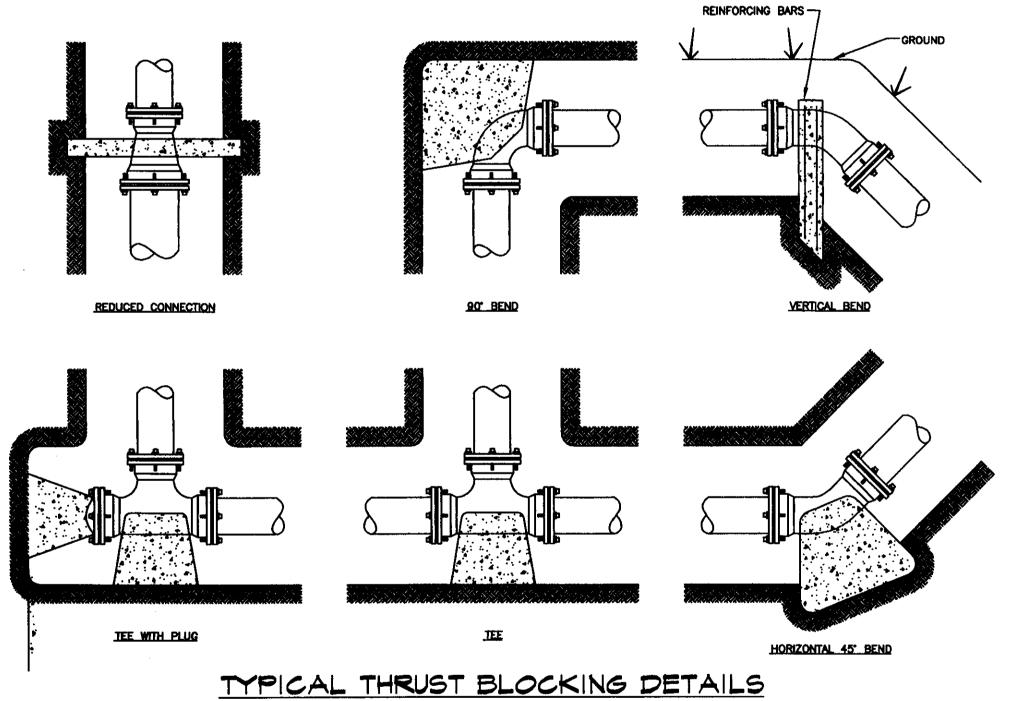
CAST IRON CLEANOUT -WITH ALL BRONZE COUNTERSUNK PLUG.

- 6" THICK X 12" SQUARE CONCRETE PAD. CROWN TO SHED WATER.

- CAST IRON TWO-WAY CLEANOUT FITTING (TYLER #003533)

- SEWER MATERIAL AND SIZE AS SHOWN ON PLANS.

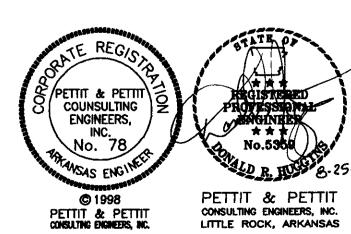
TWO-WAY CLEANOUT TO GRADE DETAIL



NOTES ON THRUST BLOCKING

ALL BLOCKING SHALL BE AGAINST HAND DUG SOIL.
 WHERE SOIL CONDITIONS MAKE IT NECESSARY TO POUR CONCRETE BLOCKING OVER JOINTS, THE ENDS OF THE ADJACENT PIPES MUST HAVE A KICKER BLOCK TO RESIST MOVEMENT OT THESE JOINTS.
 WEIGHT CALCULATIONS TO BE BASED ON THRUST DUE STACTIC PRESSURE + 50% OR TEST PRESSURE, WHICHEVER IS GREATER.
 THRUST= 2 AP SIN 1/2 0 WHERE A= AREA OF PIPE P= WATER PRESSURE
 WHEN BLOCKING AGAINST PLUG, PLUG SHALL BE COVERED TO TO PREVENT BINDING OF CONCRETE.

- WHERE SHEAR BECOMES A PROBLEM PROPER REINFORCING MUST BE INSTALLED INTO THE BLOCKING.
 CLEARANCE SHALL BE A MINIMUM OF 6" BETWEEN PIPE AND OBSTRUCTION.
 CLEARANCE ON PIPES BELONGING TO OIL & GAS COMPANIES SHALL BE 18" UNLESS SPECIAL PERMISSION IS GIVEN BY THESE COMPANIES.



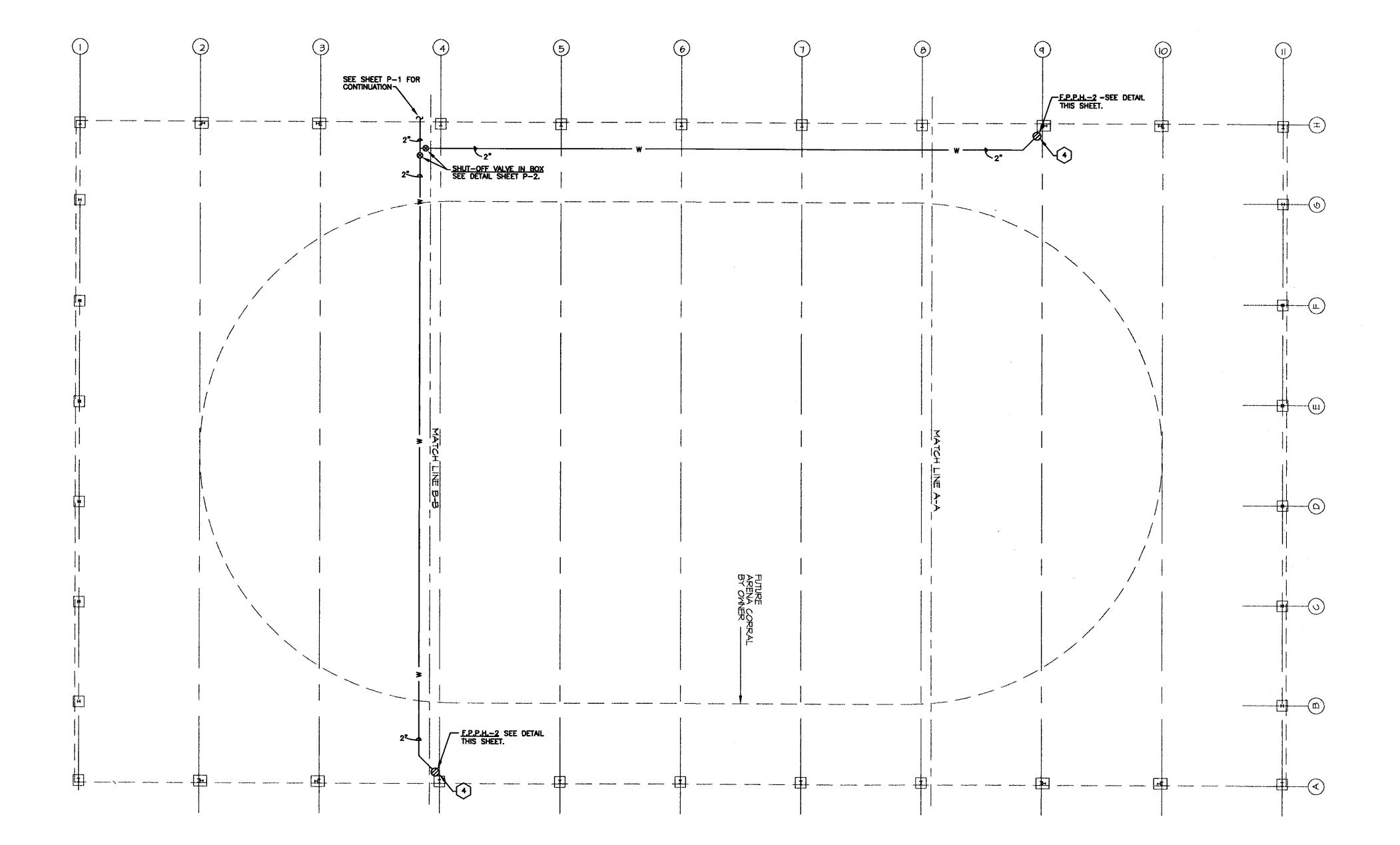
Sheet Name UTILITY DETAILS-PLUMBING/ FIRE PROTECTION 8-14-98 Drawn By

PLANNING

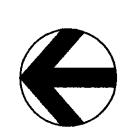
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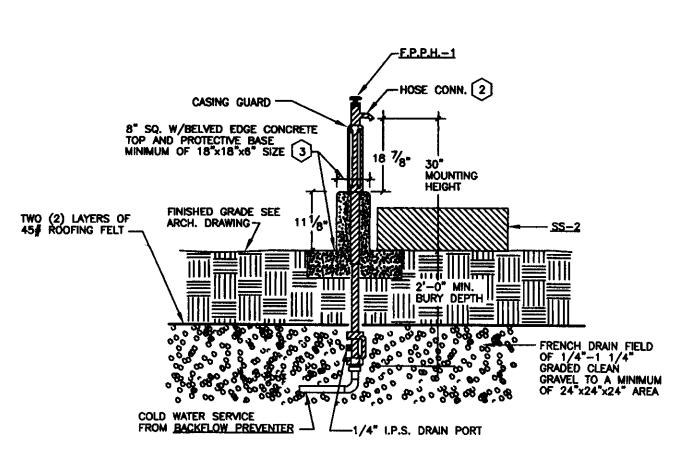
KEYED NOTES

- REFERENCE GENERAL NOTES & LEGEND ON SHEET P-1.
- 2 HOSE CONNECTION SIZE FOR FPPH IN OUTDOOR ARENA SHALL BE 2" SIZE.
- MAINTAIN POUR OF CONCRETE BASE FOR FPPH AT MINIMUM DISTANCE FROM STRUCTURE.
- PROVIDE BACKFLOW PREVENTER
 (FEBCO MODEL 765 PRESS. VAC.
 BREAKER ASSEMBLY) MOUNTED ON
 STRUCTURE ABV. FIXTURE IN WTR.
 SUPPLY.

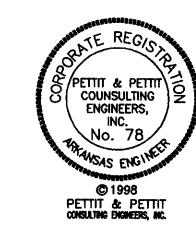


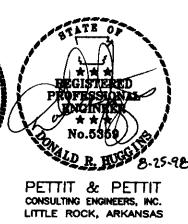
FLOOR PLAN -'OUTDOOR ARENA'-PLUMBING SCALE: 1/16"=1'-0"





FREEZE PROOF POST HYDRANT DETAIL





Sheet Name
FLOOR PLAN-'OUTDOOR'PLUMBING
Project No. Date:
8-14-98
Drawn By File No.
Sheet No.

P=3

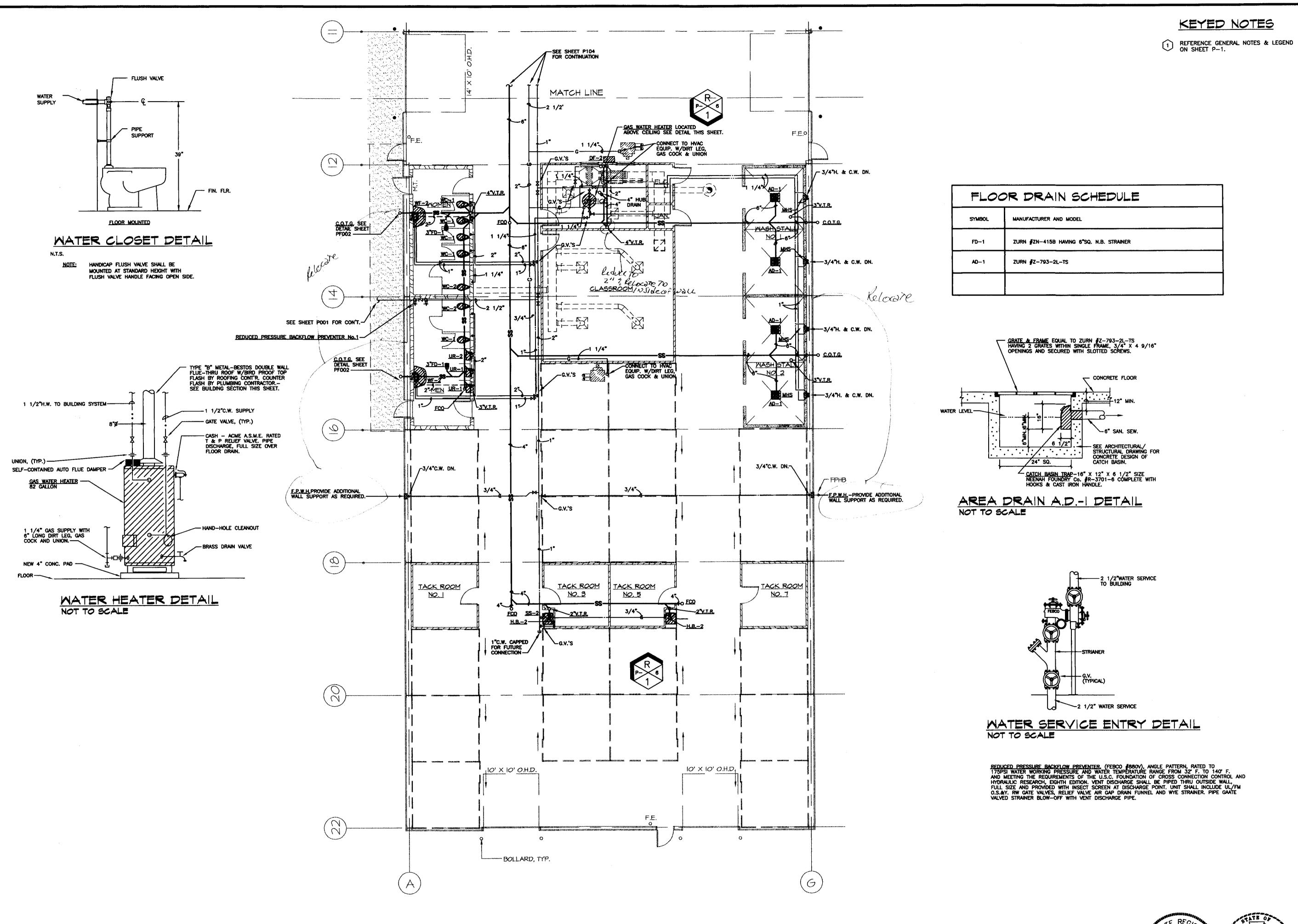
FAX (501) 933-6988 JONESBORO, ARKANSAS

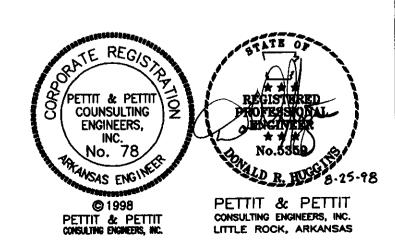
PH. (501) 933-6 256 SOUTHWEST

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PH. 256

Sheet Name FLOOR FLAN-SOUTH END'-FLUMBING

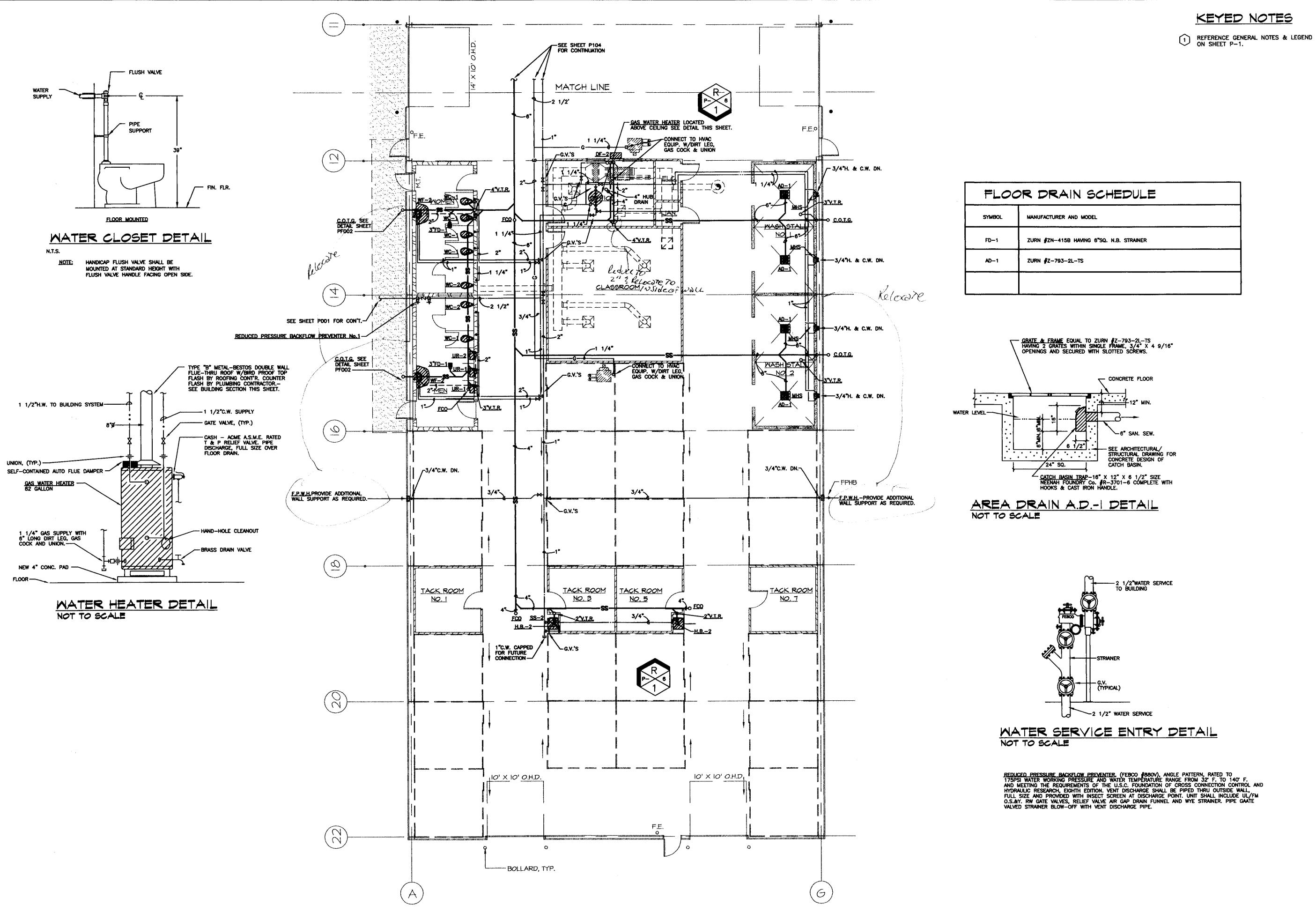
8-14-98

Project No.

Drawn By

FLOOR PLAN - 'SOUTH END'-PLUMBING

SCALE: 1/8"=1'-0"



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Sheet Name FLOOR FLAN-BOUTH END'-PLUMBING Project No. Drawn By Sheet No.

8-14-98

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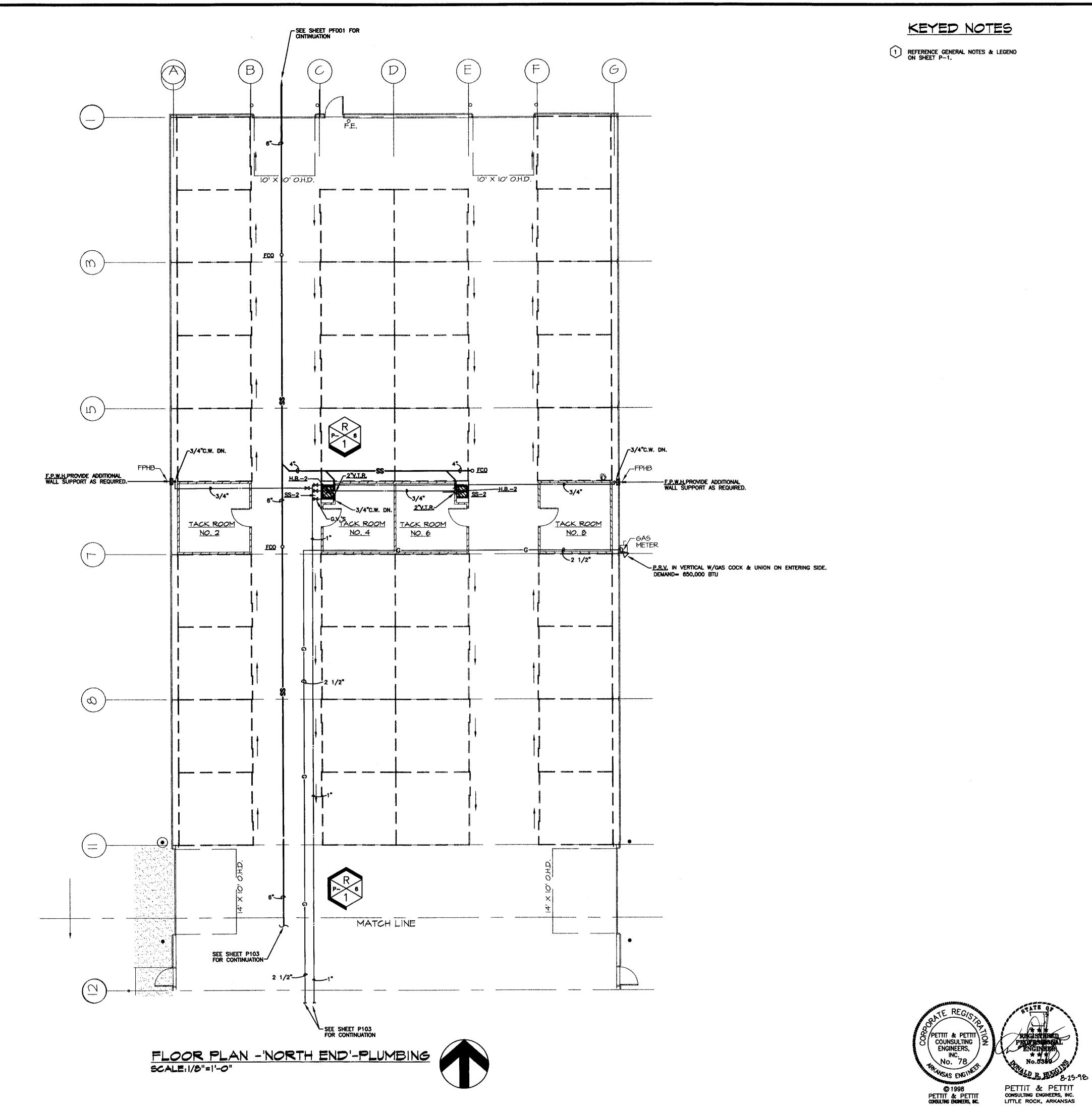
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FLOOR PLAN - 'SOUTH END'-PLUMBING

SCALE: 1/8"=1'-0"



Revisions

HE CAHOON FIRM, P. A
ARCHITECTURE AND PLANNING
PH. (501) 933-6983 FAX (501) 933-6988
256 SOUTHWEST DRIVE JONESBORO, ARKANSAS

STUDENT BARN
KANSAS STATE UNVERSITY

Sheet Name

Sheet Name
PLOOR PLAN-NORTH END'PLUMBING

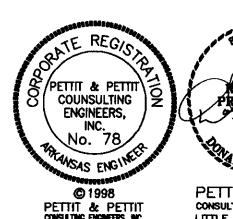
Project No.

Date:
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File No.

RISER DIAGRAMS
NOT TO SCALE



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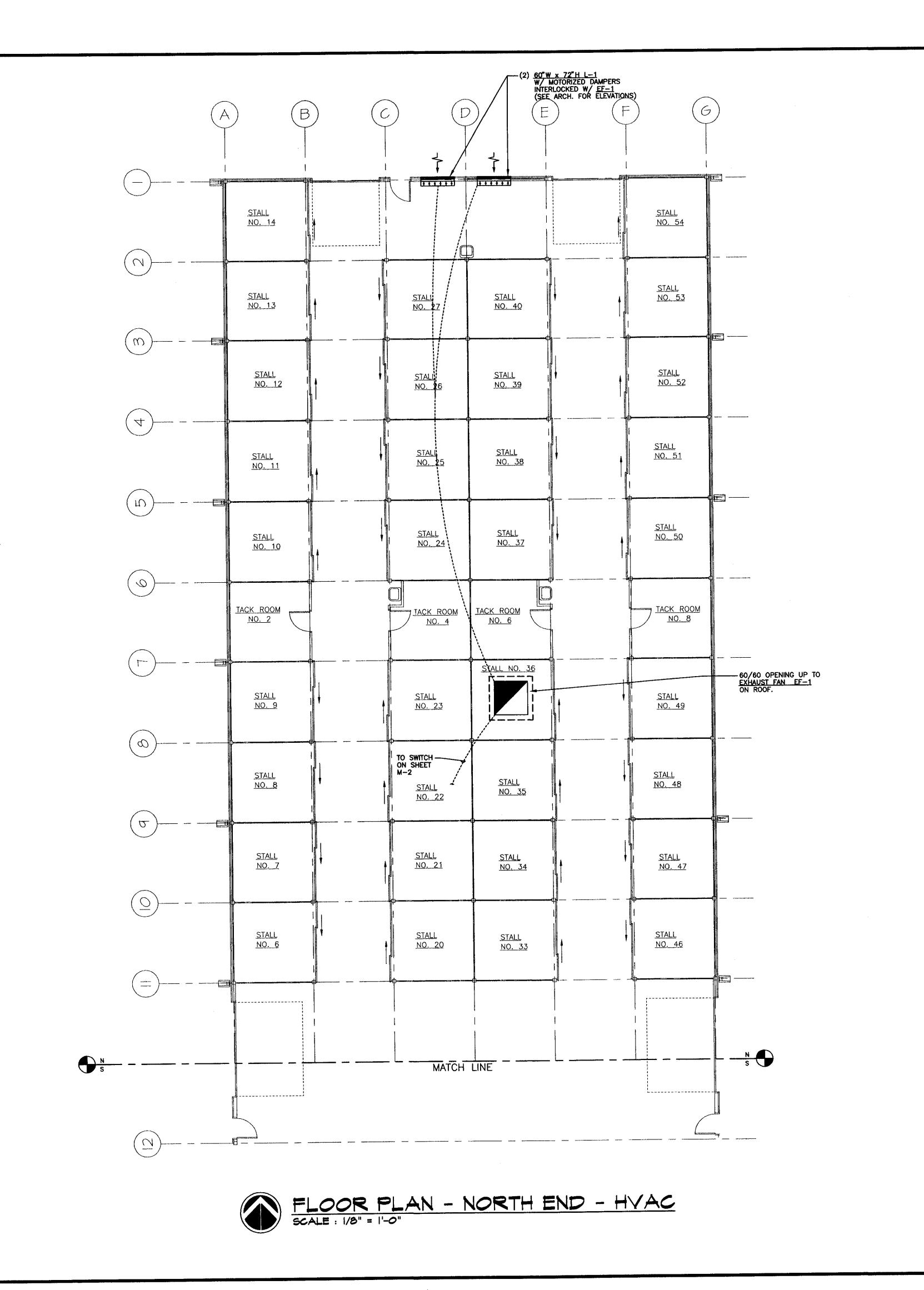
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PH. (501) 933-6993 256 SOUTHWEST DRIVE

Project No. Date: 8-14-48 Drawn By

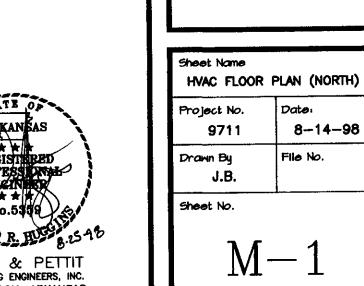


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STATE UNIVERSITY

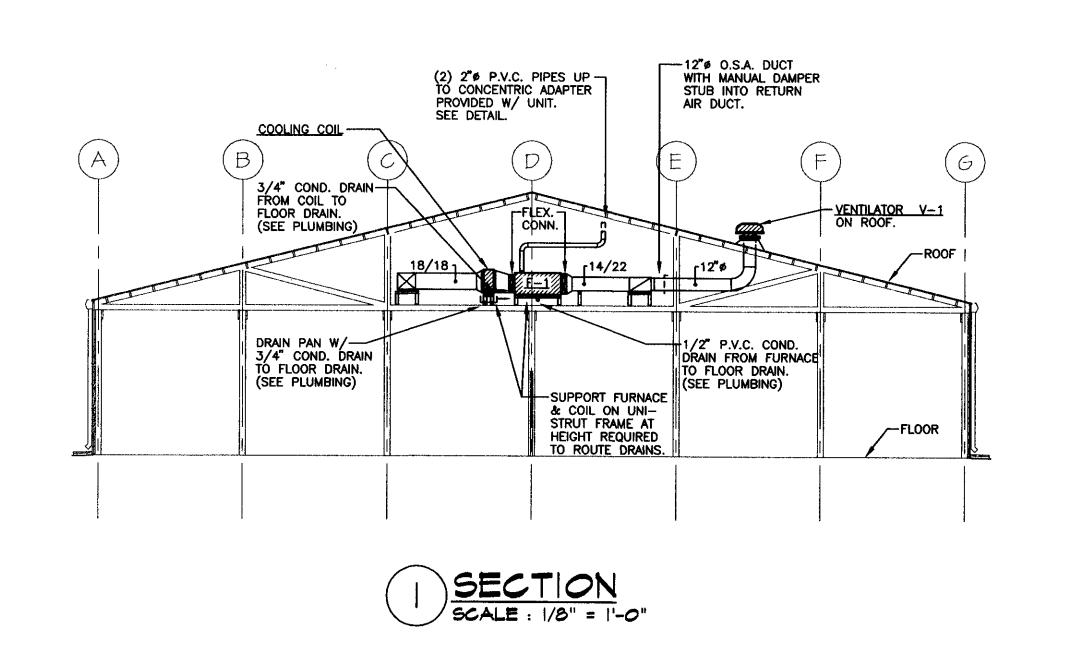
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AND PLANNING

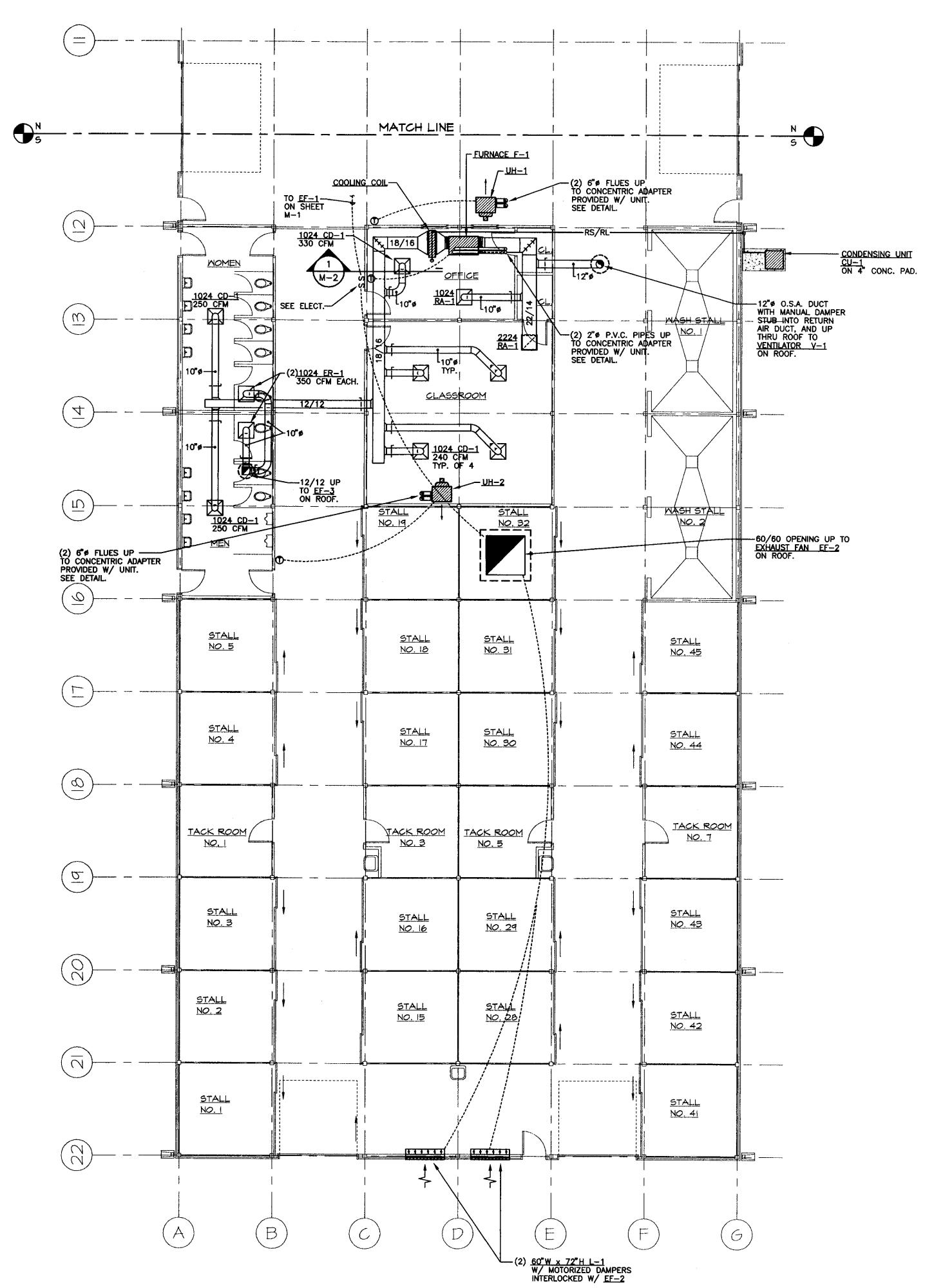
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Sheet Name
HVAC FLOOR PLAN (SOUTH)
Project No.
9711
Drawn By
J.B.
File No.

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Sheet No.

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CONSULTING ENGINEERS, INC.
LITTLE ROCK, ARKANSAS

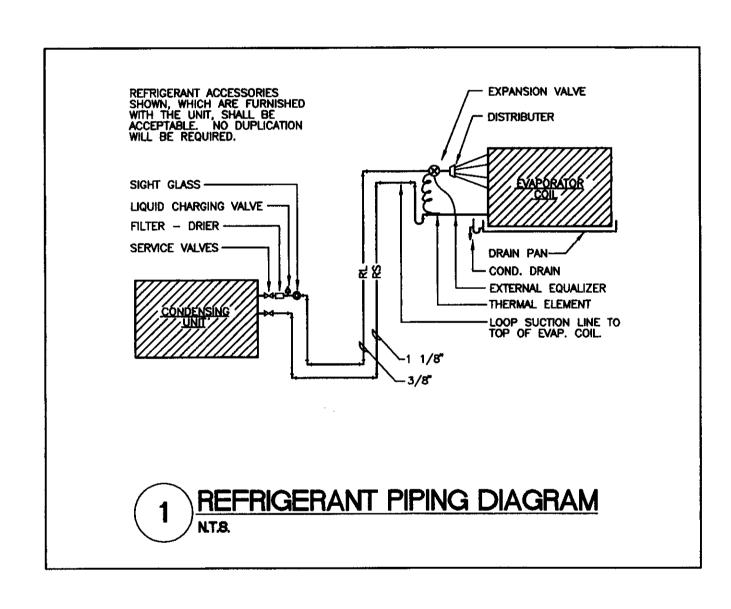
FLOOR PLAN - SOUTH END - HVAC SCALE: 1/8" = 1'-0"

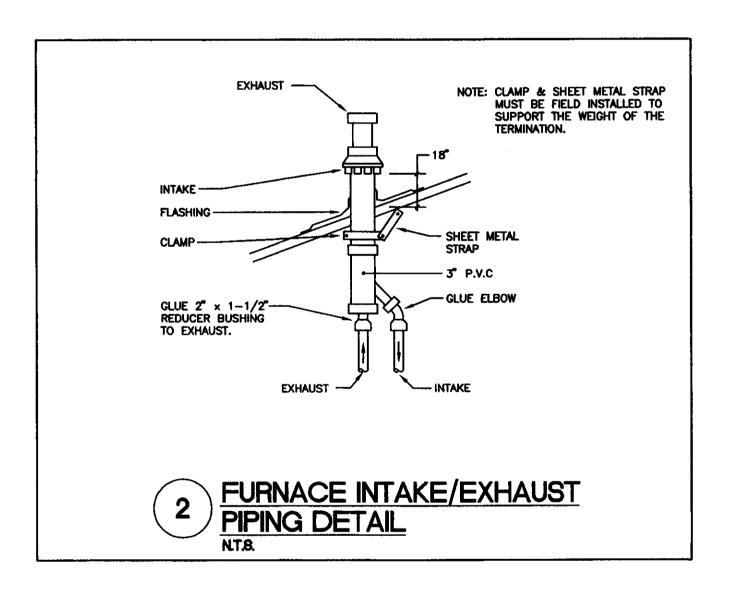
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DECIO		CONDEN	SING UNIT		E	VAPORA	TOR CO	L	COME	NED RA	TING	ELECTRICA	L	REMARKS
DESIG.	MFR/MDL	TYPE	OSA AMBIENT	SERVES	MDL.	CFM	EAT	AIR PD	TOTAL.	8EN8	88 T	VOLTS/PHASE	MCA	nemanks
CU-1	LENNOX / HS29-683	AIR COOLED	95	F-1	CH23-68	1,800	80° D.B. 67° W.B.	.20"	64.7 MBH	47.2 MBH		208 / 3ø	24.2	

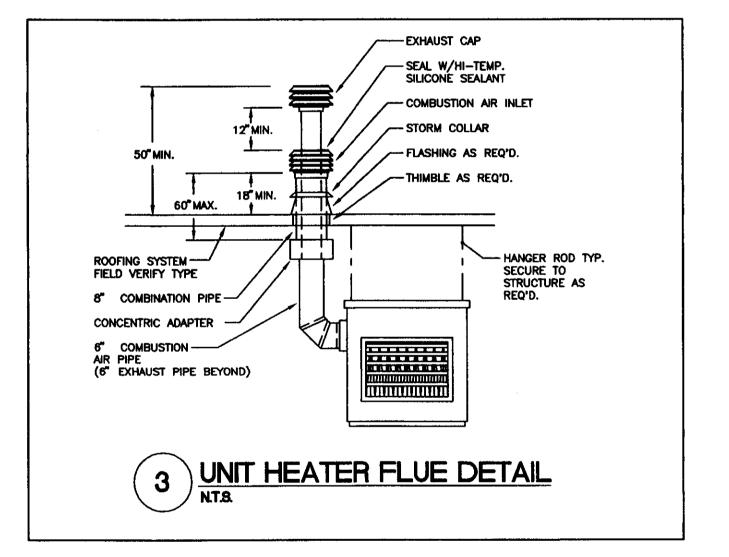
DE010		TYPE	_			FAN DAT	Ά				MOTO	R DATA		DENADVO		
DESIG.	MFR/MDL	SERVES	LOCAT.	TYPE	CFM	8.P.	RPM	DRIVE	TYPE	DIA.	SONES	RPM	BHP	HP	VOLT/PH	REMARKS
EF-1	COOK / ETE 54TE10B	BUILDING VENTILATION	ROOF	TIERED EXHAUST VENTILATOR	22,000	1/4"	518	BELT	PROPELLER	54 "	29.0	1,750 MAX.	2.90	3	208 / 3¢	PROVIDE BACKDRAFT DAMPER, DISCONNECT SWITCE & GALV. ROOF CURB.
EF-2	COOK / ETE 54TE10B	BUILDING VENTILATION	ROOF	TIERED EXHAUST VENTILATOR	22,000	1/4"	518	BELT	PROPELLER	54"	29.0	1,750 MAX.	2.90	3	208 / 34	PROVIDE BACKDRAFT DAMPER, DISCONNECT SWITCH & GALV. ROOF CURB.
EF-3	COOK / ACE-B 120C2B	TOILETS	ROOF	DOWNBLAST EXHAUST VENTILATOR	700	3/8"	988	BELT	CENTRIF.	12"	4.9	1,750 MAX.	.07	1/6	120 / 1#	PROVIDE BACKDRAFT DAMPER, DISCONNECT SWITC & GALV. ROOF CURB.

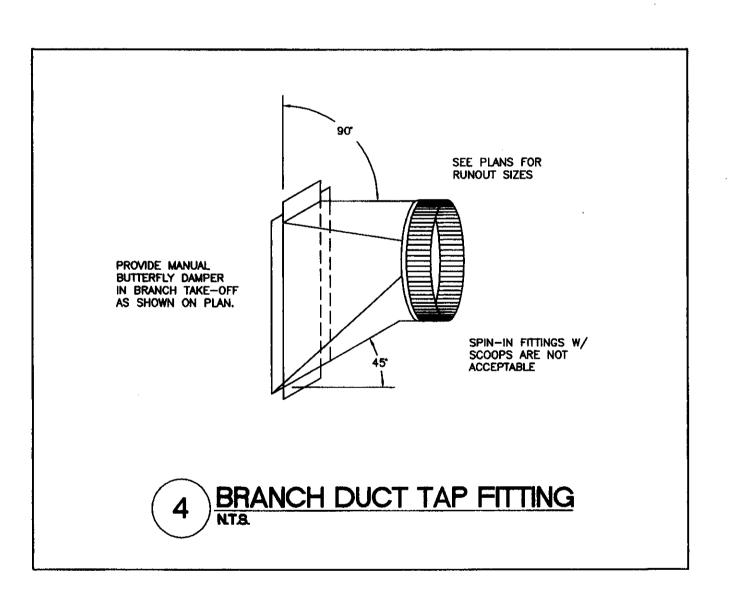
DESIG.	MFR./MDL.	TYPE	FACE SIZE	FINISH	FREE AREA	ACCESS.	REMARKS
CD-1	TUTTLE & BAILEY / PB	PERF. FACE CEILING SUPPLY	AS NOTED	WHITE	51%	VOLUME CONTROL	
RA-1	TUTTLE & BAILEY / PR	PERF. FACE CEILING RETURN	AS NOTED	WHITE	51%	<u></u>	
ER-1	TUTTLE & BAILEY / PR	PERF. FACE CEILING EXHAUST	AS NOTED	WHITE	51%	VOLUME CONTROL	
L-1	AMERICAN WRMNG. & VENT./ LE-21	INTAKE LOUVER	AS NOTED	PRIME COAT			W/ FLANGED FRAME

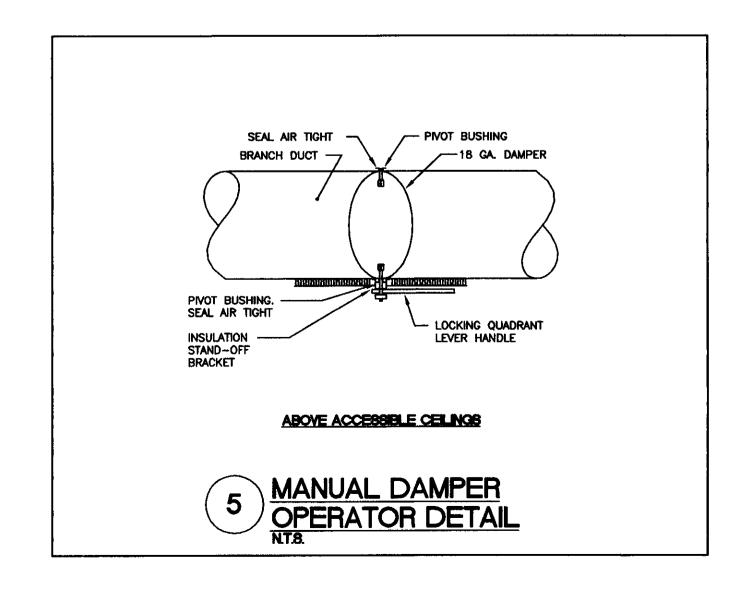
DESIG.	MFR/MDL		I TVDE !		FUEL	FUEL	FUEL	FUEL	أهد		PUEL	FUEL !	FUEL	FUEL	evel 1	INPUT		PUT OUTPUT		TEMP.	MOTOR DATA		DEMARKO
		SERVES	TYPE	CFM	FUEL	INPUL	OUIPUI	OUIPUI	OUIPUI	RISE	HP	VOLT/PH	REMARKS										
UH-1	REZNOR / SCA 150	BARN	SEPARATED COMBUSTION	1,850	NAT. GAS	150 MBH	118.5 MBH	60°	1/6	120/10	PROVIDE T-STAT &: VERTICAL VENT TERMINAL / COMBUSTION AIR INLET												
UH-2	REZNOR / SCA 150	BARN	SEPARATED COMBUSTION	1,850	NAT. GAS	150 MBH	118.5 MBH	60	1/6	120/1ø	PROVIDE T-STAT & VERTICAL VENT TERMINAL / COMBUSTION AIR INLET												

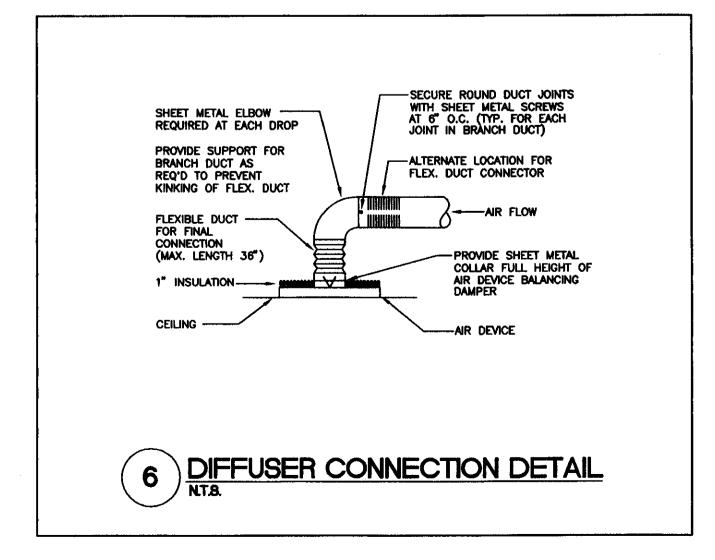


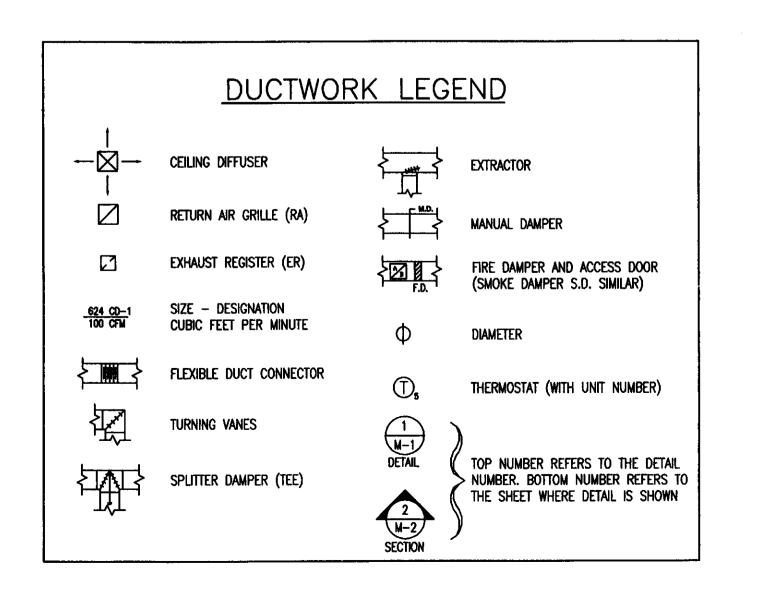


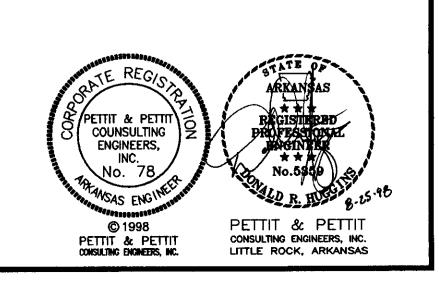












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STUDENT BARN
ARKANSAS STATE UNIVERSITY
JONESBORO ARKANSAS

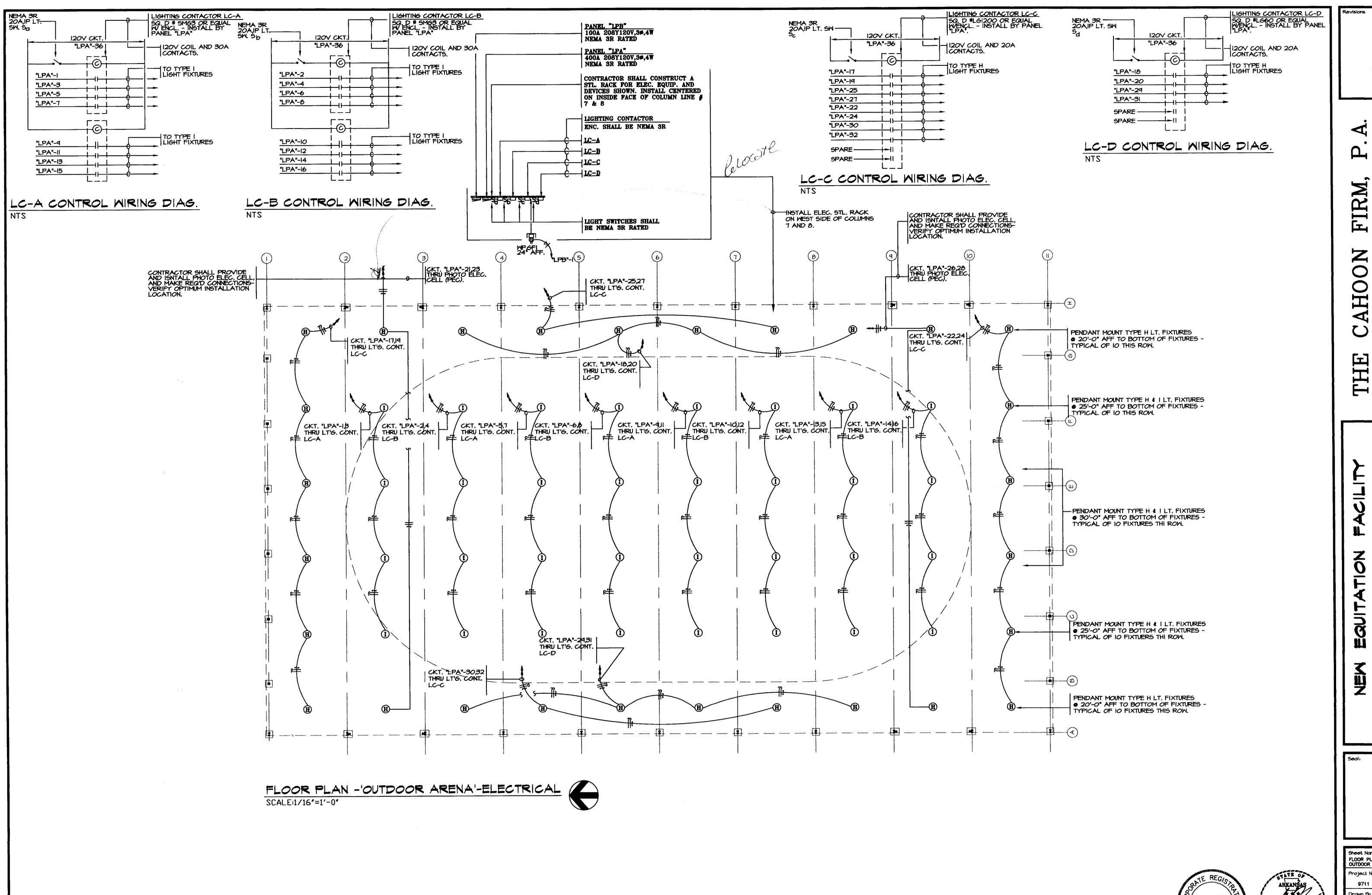
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HVAC SCHEDULES & DETAILS

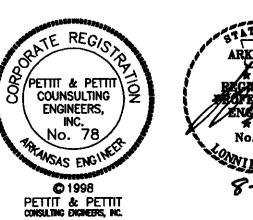
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9711 8-14-98

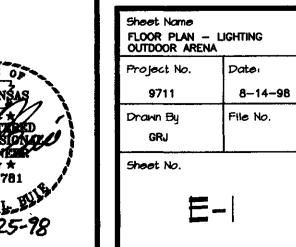
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J.B.

M-3

Sheet No.







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(501) 933-6993 SOUTHWEST DRIV

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